



10641 Highway 36
Covington, GA
30014

www.sunbeltbuilders.com

t 770.786.3031
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JACKSON COUNTY AIRPORT TERMINAL BUILDING

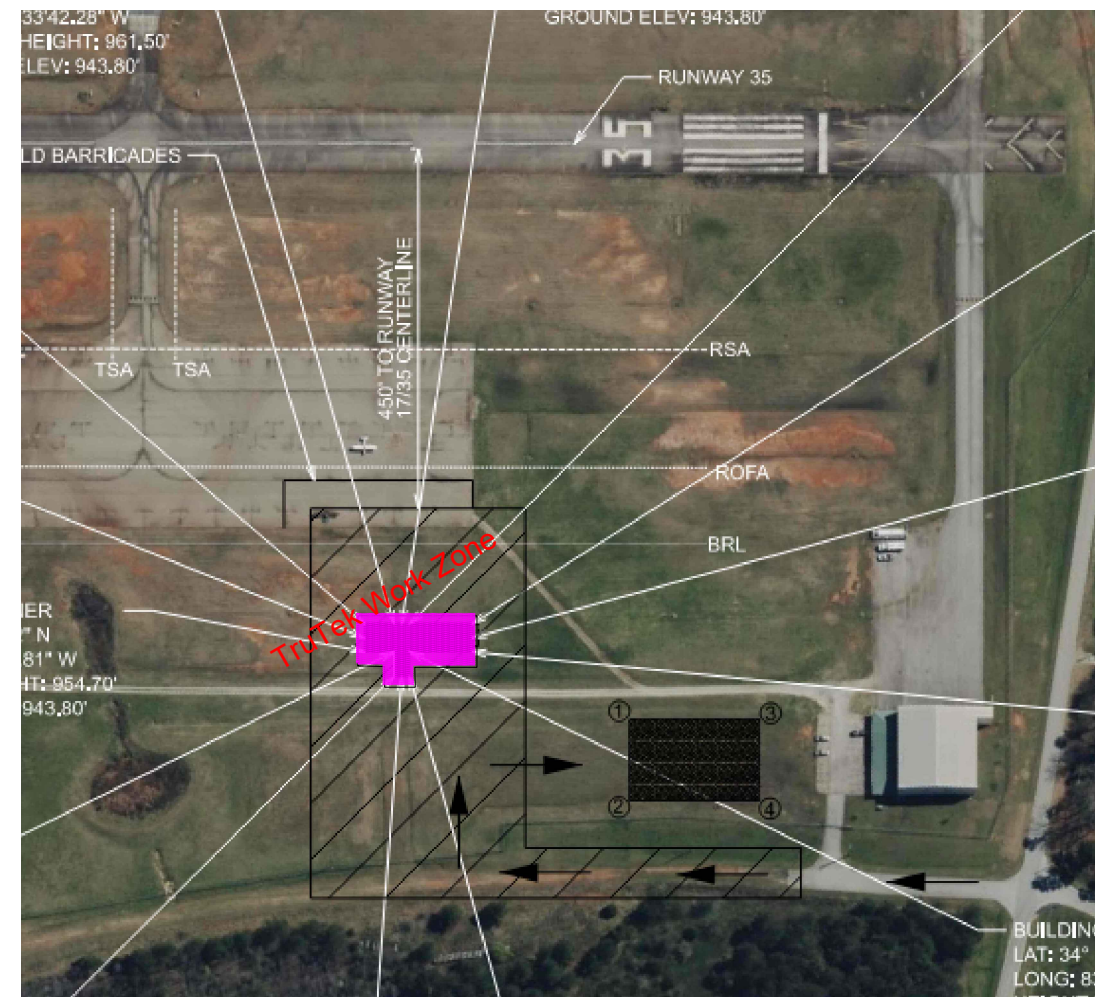
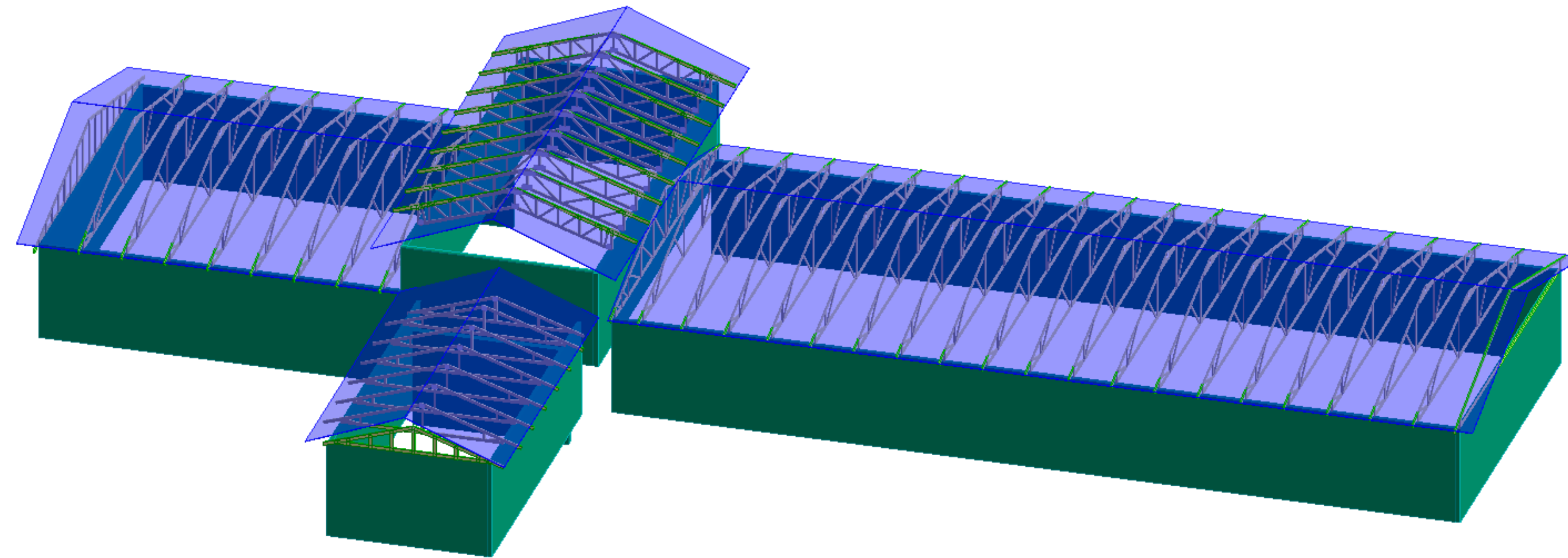
TruTek Framing Systems
4001 Hwy 153
Greenville, SC 29611
ph: 864-704-9315
Dalton Parker
dparker@trutek-fs.com

05-3000 Structural Steel, Metal Decking & Truss
As-built

NAME: Sunbelt Builders
ADDRESS: 10641 HWY 36
CITY: Covington
STATE: GA
ZIP: 30016
ELEPHONE (404)-644-2939
CONTACT: Mike McCrorey Jr

SHEET INDEX:

TL0.0	Cover Page
TL1.0	Truss Placement Plan
TL2.0	Sections
TL3.0	Roof Deck Layout Plan



JOB NAME:

Jackson County Airport Terminal
Jefferson, GA

LOCATION:



TruTek
Framing Systems LLC

ISSUE DATE:
02/27/2025

TBx.x

Dimension to Face
U.N.O.

TTx.x

6:12

Dimension to \mathbb{C}
U.N.O.

BPx



6:12
CEILING

1

Revision Number

1. Do not alter, cut, or notch a truss member without prior written approval by TruTek. Back charges will not be accepted, regardless of fault, without prior notification.
2. Coordinate the Layout of the trusses and truss members with all Mechanical Contractors. Avoid hanging mechanical equipment and/or piping from diagonal web members. If mechanical hangers require a hole to be drilled in the chord flange, the hole size must not exceed 3/8" diameter.
3. CFS trusses have a front and a back face. Typically all truss to truss and truss to bearing connections are applied to the front face (Unless Otherwise Noted.) The front face of the truss can be easily identified by viewing an individual truss drawing. When viewing a typical truss drawing you are looking at the front face or hard side of the truss.
4. See the supplemental CFSB1 Summary Sheet - Guide for Handling, Installing, Restraining and Bracing of Trusses for more information on the storage, handling, and erection bracing requirements. (Failure to follow these recommendations could result in severe personal injury or damage to the trusses or building.)
5. See the Bracing Layout in conjunction with the individual Truss Drawings for the location and extent of all required permanent lateral and diagonal bracing to resist truss member buckling forces.
6. Per AISI S240-23 the Building Designer is responsible for the clear specification of all design loads and their path or transfer throughout the building. TruTek is restricted to the roll of the Truss Designer as we endeavor to provide the necessary individual components to achieve the Building Designer's specified design for the trusses.
7. TruTek shall supply or perform only the items or services in the agreed upon contract, proposal, or statement of work. Any items or work designated as "by others" is outside of TruTek's current scope.

REVISIONS:

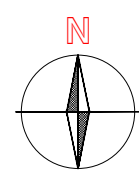
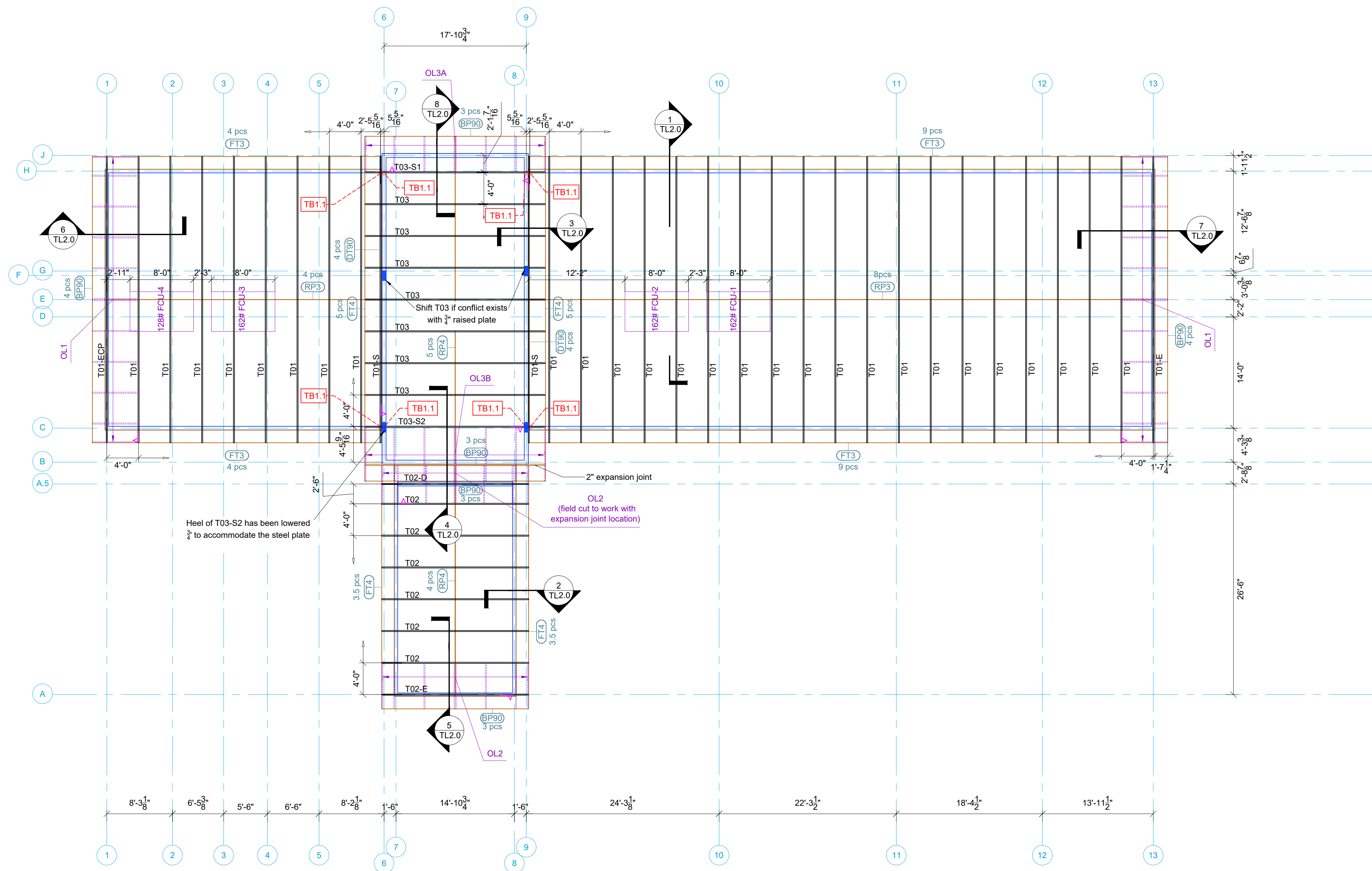
TruTek Job# 24188

DRAWN BY: ADT

APPROVED BY: JM

SHEET:

TL0.0



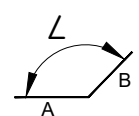
Truss Placement Plan

Structural Connection Information
(see submittal book for details)

Typical truss to bearing connection U.N.O.
(See highlighted rows on sheet TB1.0)

Direction of Hard Face

BENT PLATE SCHEDULE (figured with 6" overlaps)								
QTY	LABEL	Ga.	LENGTH	LEG (A)	LEG (B)	L°	PROFILE	DESCRIPTION
12	RP3	14	10'-0"	4"	4"	152	^	Ridge
9	RP4	14	10'-0"	4"	4"	144	^	Ridge
17	FT4	14	10'-0"	3"	3"	72	✓	Fascia-Top
26	FT3	14	10'-0"	4"	4"	76	✓	Fascia-Top
21	BP90	14	10'-0"	3"	3"	90	L	Rake
8	OT90	14	10'-0"	4"	4"	90	L	Deflection Track
40	CP11	44	4'-0"	0"	0"	400		Clear Distance



JOB NAME:

Jackson County Airport Terminal

LOCATION:

Jefferson, GA



ISSUE DATE:

02/27/2025

REVISIONS:

TruTek Job# 24188

DRAWN BY: ADT

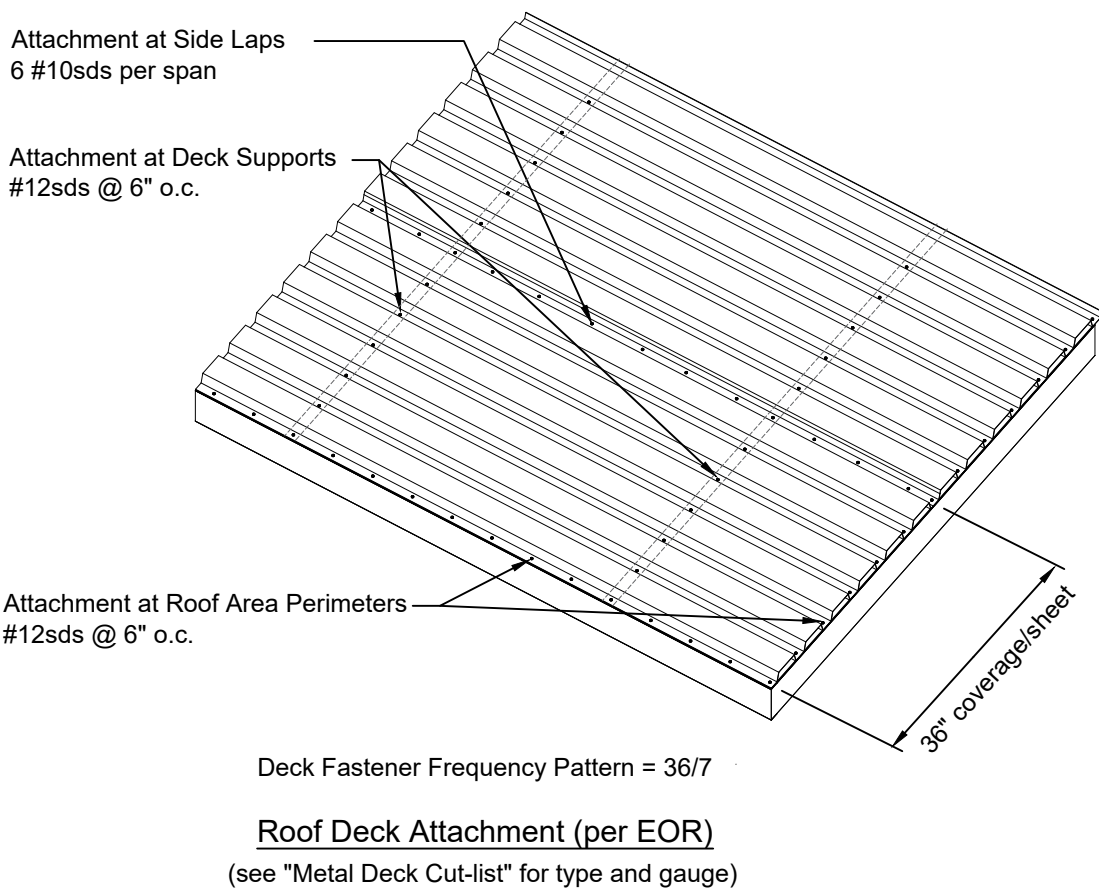
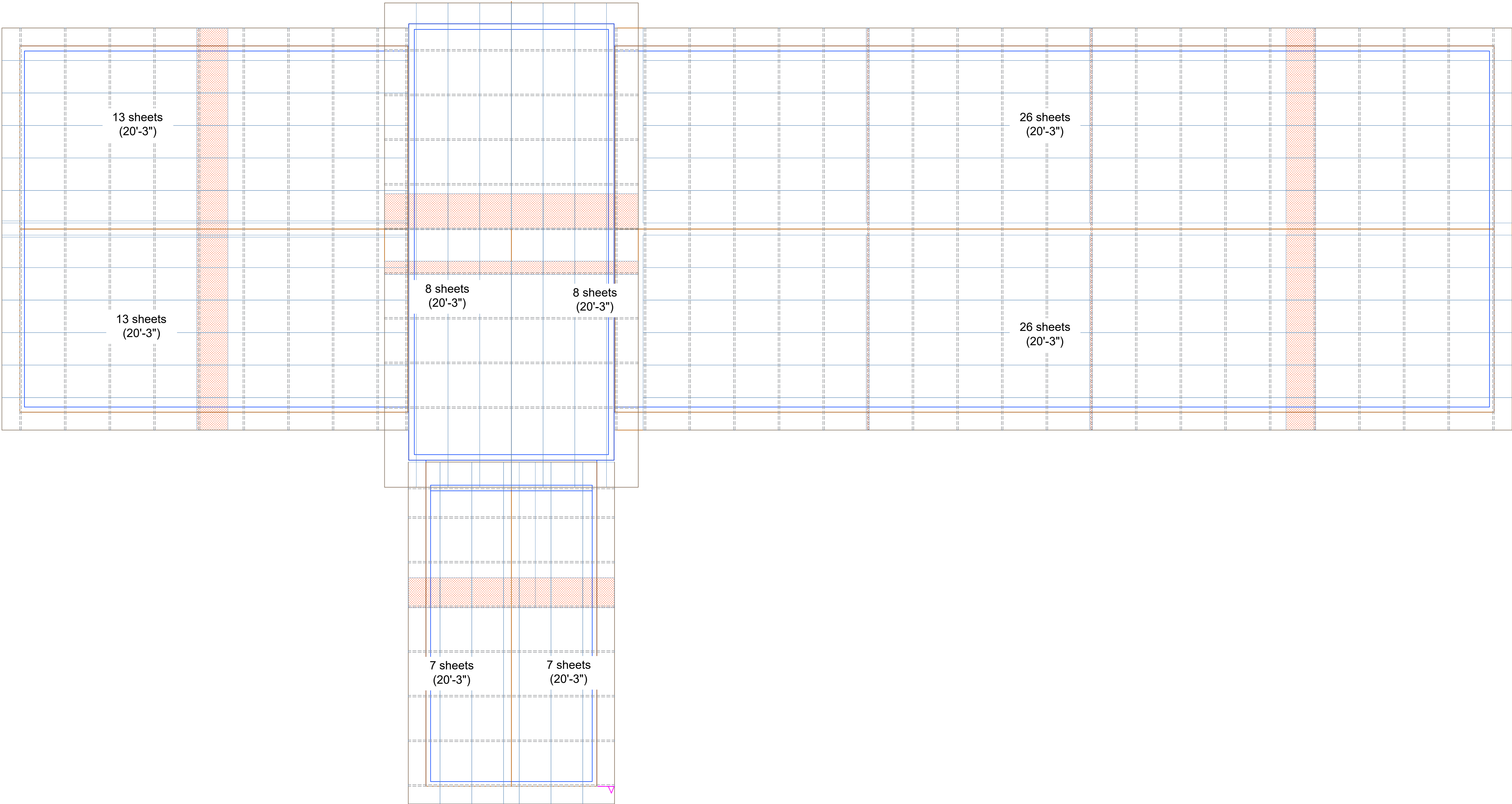
APPROVED BY: JM

SHEET:

TL1.0



TL2.0



Metal Deck Layout Plan

All connections of Deck to roof framing to be per EOR's specifications.
(see Note F. under "Steel Roof Deck" notes on S0.03 summarized below)
#12sds at supports, #10sds at side laps (hex heads)
36/7 Pattern at supports, 6 connections per span at side laps, 6" o.c. at roof area edges.

Metal Deck Cut-List						
QTY	Depth	Type	Ga.	Galv.	ksi	LENGTH
108	1.5"	B	22	G90	50	20'-3"

PAGE KEY

SHEET OF METAL DECK

TRUSS BELOW DECK

SPLICE/END LAP OF METAL DECK

(Splice Location For Illustration Purposes Only)

JOB NAME:

Jackson County Airport Terminal
Jefferson, GA

LOCATION:



ISSUE DATE:

02/27/25

REVISIONS:

1 04/25/25

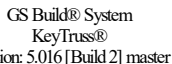
TruTek Job# 24188

DRAWN BY: ADT

APPROVED BY: JM

SHEET:

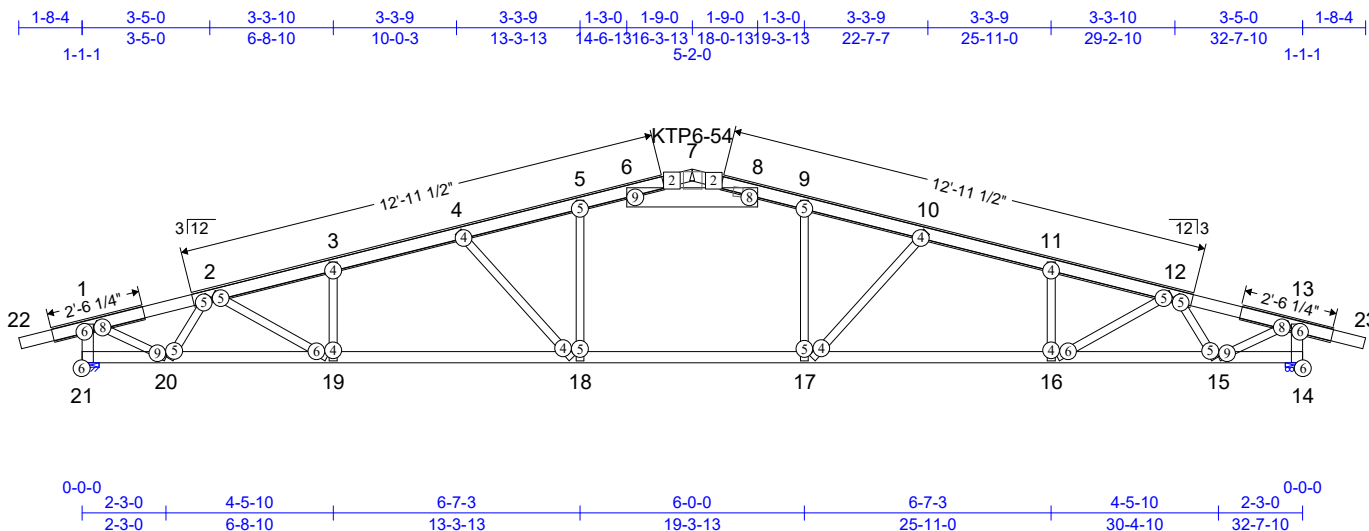
TL3.0



4001 Hwy 153
Greenville, SC 29611
864 999 2020

Truss: T01
JobName: 24188
JobID: Jackson County Airport
Date: 4/25/2025 12:37:51 PM
System: KeyTRUSS 6.113
Page: 1 of 2
Report: Eng Plot

SPAN	PITCH	QTY	OHL	OHR	PLYS	SPACING	WGT/PLY
32-7-10	3/12	27	1-8-4	1-8-4	1	48 in	223.3 lbs



Circles indicate fastener count in webs. Squares indicate fastener count in chords. "Fasteners" indicates the number of #10 SDS (AISI) fasteners required at one end of the member. Each value indicates the number of fasteners required. Where connection plates are called out on this drawing, a plate is required. Refer to General Notes for further clarification. Allowable shear per fastener is calculated per the AISI S100. Maintain fastener spacing at 9"16" min. Maintain fastener edge margin at 9"16" min for each sheet of steel connected.

TC: 0935(5-6)
BC: 0924(17-18)
Web: 0974(6-8)

TL: 0.59in
LL: 0.42in
Cant/OHTL: 0.1 inUP
Cant/OHLL: 0.07 inUP
Horz TL: 0.2 in

L/
L/ 646
L/ 911
2L/ 417
2L/ 587

(loc)
(16-17)
(16-17)
13
13
23

L/240
L/360
2L/120
2L/120

JT	Type	Brg Combo	Brg Width	Max React	Grav Uplift	MWFRS Uplift	Max Uplift	Max Horiz
21	Pin (WL)	1	5.5 in	4912 lbs				679 lbs
14	HRoll (WL)	1	5.5 in	4912 lbs				

TChd	362SI62-68(50ksi)						TChd Bracing: Sheathed	
BChd	362SI62-68(50ksi)						BChd Bracing: 108 in	
Webs	250SI62-33(50ksi)	except:						
1-21	250SI62-54(50ksi)	2-20	250SI62-43(50ksi)		12-15	250SI62-43(50ksi)	13-14	362SI62-54(50ksi)
1-20	250SI62-54(50ksi)	6-8	600TRK162-68(50ksi)		13-15	250SI62-54(50ksi)		

TChd Bracing: Sheathed
BChd Bracing: 108 in

12-15	250S162-43 (50 ksi)
13-15	250S162-54 (50 ksi)

13-14 362S162-54(50ksi)

Bold lines indicate track reinforcement is required on the hard side of the cee. Track shall match gauge/depth of cee material.

Track shall be attached with fasteners through track into cee using:

1 row 12" o.c. through each flange minimum 3 fasteners per row

1 row 12" o.c. through the web minimum 3 fasteners per row

At truss joints, fasteners that connect webs to chords may be counted as track reinforcement fasteners.

1) This truss has been designed in accordance with IBC - 2018.

2) This truss has been designed for the effects due to wind loads in accordance with ASCE7 - 16, $V = 110$ mph, Exposure C. The building is Gable with Risk Category II, Mean Roof Height 15 ft, Overall Building Dimensions of 67 ft x 131 ft, Enclosure Class: Enclosed, CC Zone Width 6 ft The left end vertical has been exposed to wind. The right end vertical has been exposed to wind. This truss is not an End Zone Truss. This truss is in a Hurricane Prone Region.

3) This truss has been designed for the effects of balanced, minimum and unbalanced snow loads in accordance with ASCE7 - 16, using a ground snow load of 5 psf and terrain category B. The exposure condition is Fully Exposed. The thermal condition is All Others ($C_t = 1.0$) with building category II ($I = 1.0$). The roof ventilation is Unknown with Roof R-Value Unknown. The roof slope is 3/12 and the surface type is All Others. This truss has not been designed to account for the effects of ice dams forming at the eaves.

Load Case Lr1: Std Live Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	Cont		Down	Proj	20 psf	20 psf	48 in
Bot	Cont		Down	Proj	30 psf	30 psf	48 in

Load Case D1: Std Dead Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	Cont		Down	Rake	12 psf	12 psf	48 in
Bot	Cont		Down	Rake	8 psf	8 psf	48 in

User-defined Load Case D2: Mech

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	Cont		Down	Rake	12psf	12psf	48 in
Bot	Cont		Down	Rake	8psf	8psf	48 in

Point Loads

Member	Location	Direction	Load	Trib Width
Top	16-74	Down	50lbs	48in
Top	18-1-8	Down	50lbs	48in
Top	14-6-2	Down	50lbs	48in
Top	16-0-10	Down	50lbs	48in

[illegible]

4/25/2025

ALL GENERAL NOTES OF THIS DRAWING PACKAGE SHALL BE CONSIDERED AS AN INTEGRAL PART OF THIS COMPONENT DESIGN DOCUMENT. NOTE THAT THE PROFESSIONAL ENGINEER'S SEAL INDICATES ONLY THAT THE TRUSS ASSEMBLY SHOWN ON THIS SHEET MEETS THE MINIMUM APPLICABLE DESIGN CRITERIA FOR THE LOADS, LOADING CONDITIONS, TRUSS MEMBER CONFIGURATIONS AND SPANS LISTED ON THIS SHEET.

Keymark Engineering, Inc.
6707 Winchester Circle, Suite 102
Boulder, Colorado 80301



GS Build® System
KeyTruss®
Version: 5.016 [Build 2] master

TruTek Framing Systems

4001 Hwy 153
Greenville, SC 29611
864 999 2020

Truss:

JobName: T01
JobID: 24188
Date: Jackson County Airport
4/25/2025 12:37:52 PM
System: KeyTRUSS 6.113
Page: 2 of 2
Report: Eng Plot

SPAN
32-7-10

PITCH
3/12

QTY
27

OHL
1-8-4

OHR
1-8-4

PLYS
1

SPACING
48 in

WGT/PLY
223.3 lbs

User-defined Load Case W10: Beam_Brace_Right

Point Loads

Member	Location	Direction	Load	Trib Width
Bot	1-40	Right	566lbs	48 in
Bot	1-40	Up	566lbs	48 in
Bot	31-5-10	Right	566lbs	48 in
Bot	31-5-10	Down	566lbs	48 in

User-defined Load Case W11: Beam_Brace_Left

Point Loads

Member	Location	Direction	Load	Trib Width
Bot	1-40	Left	566lbs	48 in
Bot	1-40	Down	566lbs	48 in
Bot	31-5-10	Left	566lbs	48 in
Bot	31-5-10	Up	566lbs	48 in

Load Combinations

#	Load Combo	Factor
1	D1	1.000
2	D2	1.000
3	D1 + Lr1	1.000
4	D2 + Lr1	1.000
5	D1 + S1	1.000
6	D1 + S2	1.000
7	D1 + S3	1.000
8	D1 + S4	1.000
9	D2 + S1	1.000
10	D2 + S2	1.000
11	D2 + S3	1.000
12	D2 + S4	1.000
13	D1 + 0.6 W3	1.000
14	D1 + 0.6 W8	1.000
15	D1 + 0.6 W9	1.000
16	D1 + 0.6 W10	1.000
17	D1 + 0.6 W11	1.000
18	D2 + 0.6 W3	1.000
19	D2 + 0.6 W8	1.000
20	D2 + 0.6 W9	1.000
21	D2 + 0.6 W10	1.000
22	D2 + 0.6 W11	1.000
23	D1 + 0.45 W3 + 0.75 Lr1	1.000
24	D1 + 0.45 W10 + 0.75 Lr1	1.000
25	D1 + 0.45 W11 + 0.75 Lr1	1.000
26	D2 + 0.45 W3 + 0.75 Lr1	1.000
27	D2 + 0.45 W10 + 0.75 Lr1	1.000
28	D2 + 0.45 W11 + 0.75 Lr1	1.000
29	D1 + 0.45 W3 + 0.75 S1	1.000
30	D1 + 0.45 W3 + 0.75 S2	1.000
31	D1 + 0.45 W3 + 0.75 S3	1.000
32	D1 + 0.45 W3 + 0.75 S4	1.000
33	D1 + 0.45 W10 + 0.75 S1	1.000
34	D1 + 0.45 W10 + 0.75 S2	1.000
35	D1 + 0.45 W10 + 0.75 S3	1.000
36	D1 + 0.45 W10 + 0.75 S4	1.000
37	D1 + 0.45 W11 + 0.75 S1	1.000
38	D1 + 0.45 W11 + 0.75 S2	1.000
39	D1 + 0.45 W11 + 0.75 S3	1.000
40	D1 + 0.45 W11 + 0.75 S4	1.000
41	D2 + 0.45 W3 + 0.75 S1	1.000
42	D2 + 0.45 W3 + 0.75 S2	1.000
43	D2 + 0.45 W3 + 0.75 S3	1.000
44	D2 + 0.45 W3 + 0.75 S4	1.000
45	D2 + 0.45 W10 + 0.75 S1	1.000
46	D2 + 0.45 W10 + 0.75 S2	1.000
47	D2 + 0.45 W10 + 0.75 S3	1.000
48	D2 + 0.45 W10 + 0.75 S4	1.000
49	D2 + 0.45 W11 + 0.75 S1	1.000
50	D2 + 0.45 W11 + 0.75 S2	1.000
51	D2 + 0.45 W11 + 0.75 S3	1.000
52	D2 + 0.45 W11 + 0.75 S4	1.000
53	0.6 D1 + 0.6 W1	1.000
54	0.6 D1 + 0.6 W2	1.000
55	0.6 D1 + 0.6 W4	1.000
56	0.6 D1 + 0.6 W10	1.000
57	0.6 D1 + 0.6 W11	1.000
58	0.6 D2 + 0.6 W1	1.000
59	0.6 D2 + 0.6 W2	1.000
60	0.6 D2 + 0.6 W4	1.000
61	0.6 D2 + 0.6 W10	1.000
62	0.6 D2 + 0.6 W11	1.000

Member Forces Summary

Table indicates: Member ID, max CSI, max axial force, (max comp force if different from max axial force)

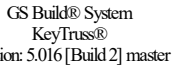
TChd	22-1	0.088	58lbs	(0lbs)	4-5	0.674	-9,289lbs	8-9*	0.924	-8,730lbs	12-13	0.860	-6,682lbs	(0lbs)		
	1-2	0.858	-6,682lbs		5-6*	0.935	-8,730lbs	9-10	0.674	-9,289lbs	13-23	0.088	58lbs			
	2-3	0.670	-10,090lbs		6-7*	0.078	-389lbs	10-11	0.673	-9,969lbs						
	3-4	0.673	-9,969lbs		7-8*	0.078	-389lbs	11-12	0.670	-10,090lbs						
BChd	14-15*	0.104	340lbs	(-340lbs)	16-17	0.903	9,695lbs	(0lbs)	18-19	0.903	9,695lbs	(0lbs)	20-21*	0.112	679lbs	(-679lbs)
	15-16	0.712	8,135lbs	(0lbs)	17-18	0.924	8,876lbs	(0lbs)	19-20	0.712	8,134lbs	(0lbs)				
Webs	1-21	0.811	-4,797lbs		3-19	0.067	-176lbs		9-17	0.399	1,929lbs	(0lbs)	12-15	0.799	-3,092lbs	(0lbs)
	1-20	0.898	7,206lbs	(0lbs)	4-18	0.858	-1,275lbs		10-17	0.858	-1,275lbs		13-15	0.898	7,206lbs	
	2-20	0.799	-3,091lbs		5-18	0.399	1,929lbs	(0lbs)	11-16	0.063	-167lbs		13-14	0.811	-4,797lbs	
	2-19	0.369	1,786lbs	(-95lbs)	6-8	0.974	-8,616lbs		12-16	0.369	1,786lbs	(-95lbs)				

Additional Notes:

The end of every chord segment shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6 " of the end of each chord segment.

NOTE : Web crippling calculation assume truss is fastened to support.

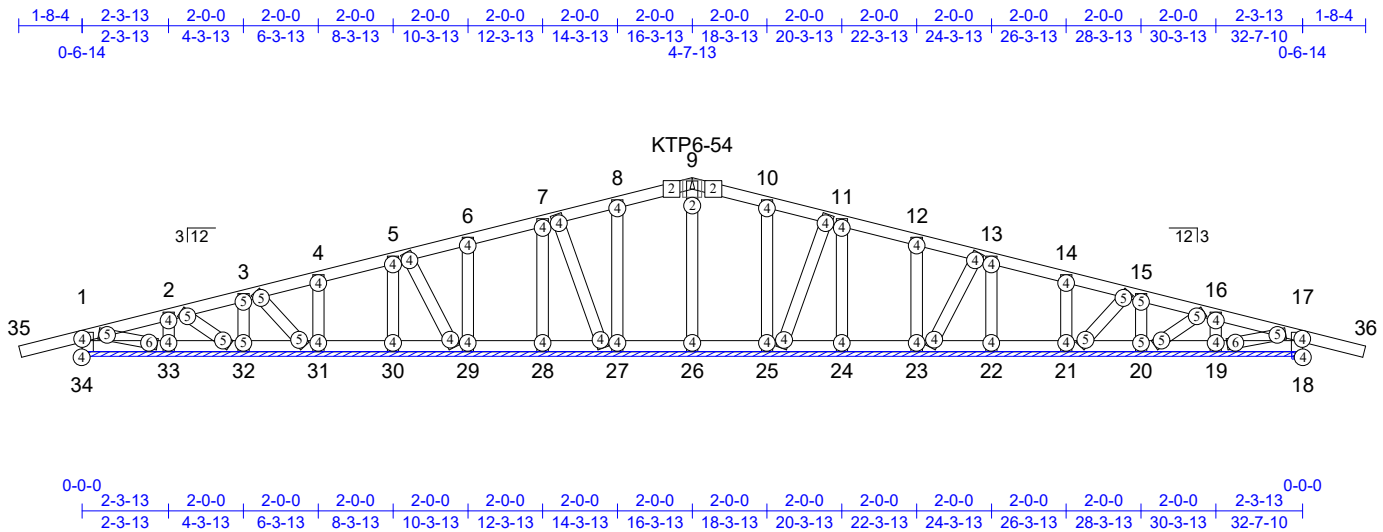
** WARNING : Web crippling prevention device required!



4001 Hwy 153
Greenville, SC 29611
864 999 2020

Truss: T01-E
JobName: 24188
JobID: Jackson County Airport
Date: 2/28/2025 6:50:45 AM
System: KeyTRUSS 6.113
Page: 1 of 2
Report: Eng Plot

SPAN	PITCH	QTY	OHL	OHR	PLYS	SPACING	WGT/PLY
32-7-10	3/12	1	1-8-4	1-8-4	1	48 in	222.8 lbs



5
5
36
36
9

Distributed Loads							
Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	Cent		PRight	Rake	300 plf	300 plf	
Bot	Cent		PLLeft	Rake	330 plf	330 plf	



Keymark Engineering, Inc.
6707 Winchester Circle, Suite 102
Boulder, Colorado 80301



GS Build® System
KeyTruss®
Version: 5.016 [Build 2] master

TruTek Framing Systems

4001 Hwy 153
Greenville, SC 29611
864 999 2020

Truss: T01-E
JobName: 24188
JobID: Jackson County Airport
Date: 2/28/2025 6:50:46 AM
System: KeyTRUSS 6.113
Page: 2 of 2
Report: Eng Plot

SPAN	PITCH	QTY	OHL	OHR	PLYS	SPACING	WGT/PLY
32-7-10	3/12	1	1-8-4	1-8-4	1	48 in	222.8 lbs

User-defined Load Case L2: Drag_Left

Distributed Loads

Member	Location1	Location2	Direction	Spread	Start Load	End Load	Trib Width
Top	Cont		PLLeft	Rake	300plf	300plf	
Bot	Cont		PRRight	Rake	330plf	330plf	

Load Combinations

#	LoadCombo	Factor
1	D1	1.000
2	D1+L1	1.000
3	D1+L2	1.000
4	D1+Lr1	1.000
5	D1+S1	1.000
6	D1+S2	1.000
7	D1+S3	1.000
8	D1+S4	1.000
9	D1+0.75L1+0.75Lr1	1.000
10	D1+0.75L2+0.75Lr1	1.000
11	D1+0.75L1+0.75S1	1.000
12	D1+0.75L1+0.75S2	1.000
13	D1+0.75L1+0.75S3	1.000
14	D1+0.75L1+0.75S4	1.000
15	D1+0.75L2+0.75S1	1.000
16	D1+0.75L2+0.75S2	1.000
17	D1+0.75L2+0.75S3	1.000
18	D1+0.75L2+0.75S4	1.000
19	D1+0.6W3	1.000
20	D1+0.6W8	1.000
21	D1+0.6W9	1.000
22	D1+0.45W3+0.75L1+0.75Lr1	1.000
23	D1+0.45W3+0.75L2+0.75Lr1	1.000
24	D1+0.45W3+0.75L1+0.75S1	1.000
25	D1+0.45W3+0.75L1+0.75S2	1.000
26	D1+0.45W3+0.75L1+0.75S3	1.000
27	D1+0.45W3+0.75L1+0.75S4	1.000
28	D1+0.45W3+0.75L2+0.75S1	1.000
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33	0.6D1+0.6W2	1.000
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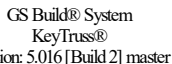
Member Forces Summary

Table indicates: Member ID,max CSI,max axial force,(max comp force if different from max axial force)

TChd	35-1	0.183	591 lbs	(-546 lbs)	5-6	0.410	-1,952 lbs	10-11	0.290	1,068 lbs	(-1,059 lbs)	15-16	0.366	-2,080 lbs		
	1-2	0.186	-1,494 lbs		6-7	0.232	-1,507 lbs	11-12	0.230	-1,493 lbs		16-17	0.172	-1,491 lbs		
	2-3	0.371	-2,089 lbs		7-8	0.292	1,081 lbs	(-1,072 lbs)	12-13	0.408	-1,938 lbs	17-36	0.183	591 lbs	(-546 lbs)	
	3-4	0.414	-2,587 lbs		8-9	0.141	-637 lbs		13-14	0.271	-2,095 lbs					
	4-5	0.272	-2,109 lbs		9-10	0.140	-623 lbs		14-15	0.413	-2,573 lbs					
BChd	18-19	0.111	-809 lbs		22-23	0.302	1,685 lbs	(-1,676 lbs)	26-27	0.177	-662 lbs	30-31	0.317	2,324 lbs	(-2,316 lbs)	
	19-20	0.168	1,295 lbs	(-1,272 lbs)	23-24	0.282	-1,640 lbs		27-28	0.203	-960 lbs	31-32	0.328	1,919 lbs	(-1,902 lbs)	
	20-21	0.332	1,943 lbs	(-1,925 lbs)	24-25	0.205	-980 lbs		28-29	0.280	-1,620 lbs	32-33	0.172	1,261 lbs	(-1,241 lbs)	
	21-22	0.318	2,345 lbs	(-2,336 lbs)	25-26	0.179	-682 lbs		29-30	0.300	1,664 lbs	(-1,656 lbs)	33-34	0.129	-706 lbs	
Webs	1-34	0.256	-920 lbs		5-30	0.181	-1,230 lbs		10-25	0.072	-416 lbs	15-21	0.229	-1,589 lbs		
	1-33	0.184	1,386 lbs	(-1,272 lbs)	5-29	0.207	-1,363 lbs		11-25	0.183	-1,079 lbs	15-20	0.229	-1,621 lbs		
	2-33	0.152	-1,096 lbs		6-29	0.059	-390 lbs		11-24	0.154	-948 lbs	16-20	0.227	-1,589 lbs		
	2-32	0.228	-1,599 lbs		7-28	0.154	-948 lbs		12-23	0.059	-390 lbs	16-19	0.154	-1,104 lbs		
	3-32	0.230	-1,631 lbs		7-27	0.183	-1,078 lbs		13-23	0.207	-1,363 lbs	17-19	0.173	1,292 lbs	(-1,198 lbs)	
	3-31	0.230	-1,592 lbs		8-27	0.072	-416 lbs		13-22	0.181	-1,230 lbs	17-18	0.264	-892 lbs		
	4-31	0.046	-318 lbs		9-26	0.044	-232 lbs		14-21	0.046	-318 lbs					

Additional Notes:

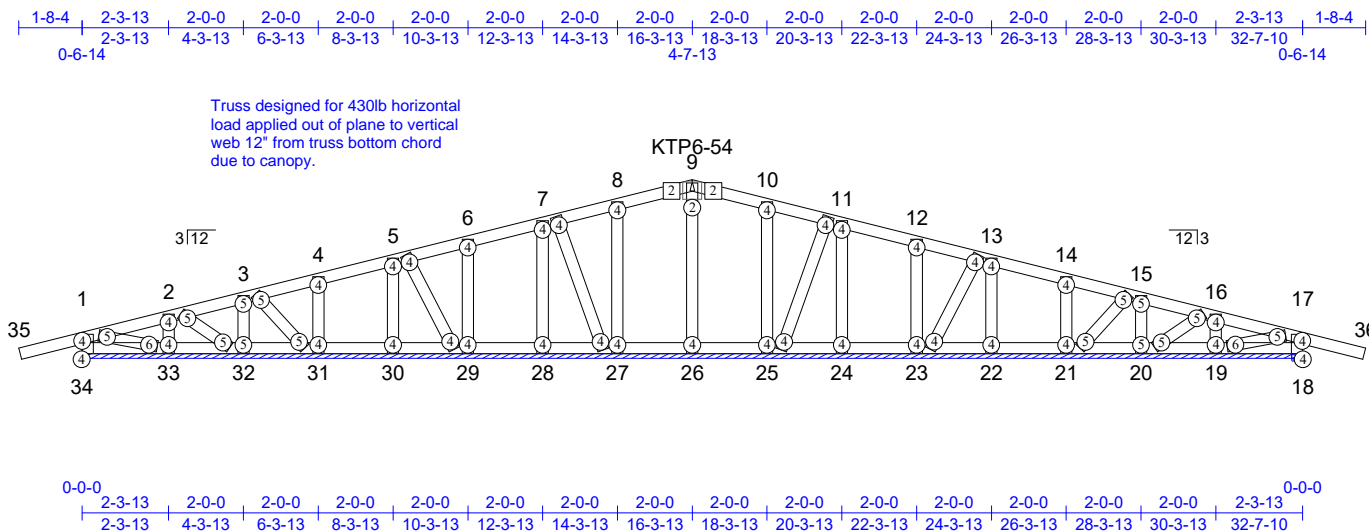
The end of every chord segment shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6 " of the end of each chord segment.
NOTE : Web crippling calculation assume truss is fastened to support.



4001 Hwy 153
Greenville, SC 29611
864 999 2020

Truss: T01-ECP
JobName: 24188
JobID: Jackson County Airport
Date: 2/28/2025 6:51:06 AM
System: KeyTRUSS 6.113
Page: 1 of 2
Report: Eng Plot

SPAN	PITCH	QTY	OHL	OHR	PLYS	SPACING	WGT/PLY
32-7-10	3/12	1	1-8.4	1-8.4	1	48 in	222.8 lbs



Circles indicate fastener count in webs. Squares indicate fastener count in chords. "Fasteners" indicates the number of #10 SDS (AISI) fasteners required at one end of the member. Each value indicates the number of fasteners required. Where connection plates are called out on this drawing, a plate is required. Refer to General Notes for further clarification. Allowable shear per fastener is calculated per the AISI S100. Maintain fastener spacing at 9/16" min. Maintain fastener edge margin at 9/16" min for each sheet of steel connected.

TC: 0.414(3-4)
BC: 0.421(21)
Web: 0.264(17-18)

TL: 0.01 in
LL: 0.01 in UP
Cant/OHTL: 0.02 in
Cant/OHLL: 0.01 in
H_{oz} TL: 0.02 in

L/999
L/999
2L/999
2L/999

5
5
36
36
9

L/240
L/360
2L/180
2L/180

JT	Type	Brg Combo	Brg Width	Max React	Grav Up/ltf	MWFRS Up/ltf	Max Up/ltf	Max Horiz
18	HRoll (WL)	1	393.625 in	287 lbs				
19	HRoll (WL)	1	393.625 in	809 lbs	-527 lbs	-351 lbs	-527 lbs	
20	HRoll (WL)	1	393.625 in	633 lbs	-305 lbs	-203 lbs	-305 lbs	
21	HRoll (WL)	1	393.625 in	980 lbs	-648 lbs	-458 lbs	-648 lbs	
22	HRoll (WL)	1	393.625 in	1,141 lbs	-840 lbs	-602 lbs	-840 lbs	
23	HRoll (WL)	1	393.625 in	1,010 lbs	-662 lbs	-468 lbs	-662 lbs	
24	HRoll (WL)	1	393.625 in	929 lbs	-643 lbs	-456 lbs	-643 lbs	
25	HRoll (WL)	1	393.625 in	859 lbs	-476 lbs	-325 lbs	-476 lbs	
26	HRoll (WL)	1	393.625 in	282 lbs				
27	HRoll (WL)	1	393.625 in	859 lbs	-476 lbs	-325 lbs	-476 lbs	
28	HRoll (WL)	1	393.625 in	929 lbs	-643 lbs	-455 lbs	-643 lbs	
29	HRoll (WL)	1	393.625 in	1,011 lbs	-662 lbs	-468 lbs	-662 lbs	
30	HRoll (WL)	1	393.625 in	1,142 lbs	-841 lbs	-603 lbs	-841 lbs	
31	HRoll (WL)	1	393.625 in	984 lbs	-653 lbs	-461 lbs	-653 lbs	
32	HRoll (WL)	1	393.625 in	645 lbs	-314 lbs	-211 lbs	-314 lbs	
33	HRoll (WL)	1	393.625 in	748 lbs	-478 lbs	-310 lbs	-478 lbs	
34	Pnn (WL)	1	393.625 in	1,027 lbs	-565 lbs	-443 lbs	-565 lbs	108 lbs
18	HRoll (WL)	1	3.5 in	707 lbs	-558 lbs	-472 lbs	-558 lbs	

[illegible]

TChd	362S200-54 (50 ksi)
BChd	362S200-54 (50 ksi)
Webs	362S200-54 (50 ksi)

TChd Bracing: 48 in
BChd Bracing: 108 in

1) This truss has been designed in accordance with IBC - 2018.

2) This truss has been designed for the effects due to wind loads in accordance with ASCE7 - 16, V = 110 mph, Exposure C. The building is Gable with Risk Category II, Mean Roof Height 15 ft, Overall Building Dimensions of 67 ft x 131 ft, Enclosure Class: Enclosed, CC Zone Width 6 ft. The left end vertical has been exposed to wind. The right end vertical has been exposed to wind. This truss is not an End Zone Truss. This truss is in a Hurricane Prone Region.

3) This truss has been designed for the effects of balanced, minimum and unbalanced snow loads in accordance with ASCE7 - 16, using a ground snow load of 5 psf and terrain category B. The exposure condition is Fully Exposed. The thermal condition is All Others ($C_t = 1.0$) with building category II ($I = 1.0$). The roof ventilation is Unknown with Roof R-Value Unknown. The roof slope is 3/12 and the surface type is All Others. This truss has not been designed to account for the effects of ice dams forming at the eaves.

Load Case Lr1: Std Live Load

Distributed Loads							
Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	Cent		Down	Proj	20 psf	20 psf	48 in
Bot	Cent		Down	Proj	0 psf	0 psf	48 in

Load Case D1: Std Dead Load

Distributed Loads							
Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	Cont		Down	Rake	12 psf	12 psf	48 in
Bot	Cont		Down	Rake	8 psf	8 psf	48 in

User-defined Load Case L1: Drag Right

Distributed Loads						
Member	Location 1	Location 2	Direction	Spread	Start Load	End Load
Top	Cont		PRight	Rake	300 plf	300 plf
Bot	Cont		PLeft	Rake	330 plf	330 plf

2/28/2025

ALL GENERAL NOTES OF THIS DRAWING PACKAGE SHALL BE CONSIDERED AS AN INTEGRAL PART OF THIS COMPONENT DESIGN DOCUMENT. NOTE THAT THE PROFESSIONAL ENGINEER'S SEAL INDICATES ONLY THAT THE TRUSS ASSEMBLY SHOWN ON THIS SHEET MEETS THE MINIMUM APPLICABLE DESIGN CRITERIA FOR THE LOADS, LOADING CONDITIONS, TRUSS MEMBER CONFIGURATIONS AND SPANS LISTED ON THIS SHEET.

Keymark Engineering, Inc.
6707 Winchester Circle, Suite 102
Boulder, Colorado 80301



GS Build® System
KeyTruss®
Version: 5.016 [Build 2] master

TruTek Framing Systems

4001 Hwy 153
Greenville, SC 29611
864 999 2020

Truss:

JobName: 24188
JobID: Jackson County Airport
Date: 2/28/2025 6:51:06 AM
System: KeyTRUSS 6.113
Page: 2 of 2
Report: Eng Plot

T01-ECP

SPAN
32-7-10

PITCH
3/12

QTY
1

OHL
1-8-4

OHR
1-8-4

PLYS
1

SPACING
48 in

WGT/PLY
222.8 lbs

User-defined Load Case L2: Drag_Left

Distributed Loads

Member	Location1	Location2	Direction	Spread	Start Load	End Load	Trib Width
Top	Cont		PLLeft	Rake	300plf	300plf	
Bot	Cont		PRRight	Rake	330plf	330plf	

Load Combinations

#	LoadCombo	Factor
1	D1	1.000
2	D1+L1	1.000
3	D1+L2	1.000
4	D1+Lr1	1.000
5	D1+S1	1.000
6	D1+S2	1.000
7	D1+S3	1.000
8	D1+S4	1.000
9	D1+0.75L1+0.75Lr1	1.000
10	D1+0.75L2+0.75Lr1	1.000
11	D1+0.75L1+0.75S1	1.000
12	D1+0.75L1+0.75S2	1.000
13	D1+0.75L1+0.75S3	1.000
14	D1+0.75L1+0.75S4	1.000
15	D1+0.75L2+0.75S1	1.000
16	D1+0.75L2+0.75S2	1.000
17	D1+0.75L2+0.75S3	1.000
18	D1+0.75L2+0.75S4	1.000
19	D1+0.6W3	1.000
20	D1+0.6W8	1.000
21	D1+0.6W9	1.000
22	D1+0.45W3+0.75L1+0.75Lr1	1.000
23	D1+0.45W3+0.75L2+0.75Lr1	1.000
24	D1+0.45W3+0.75L1+0.75S1	1.000
25	D1+0.45W3+0.75L1+0.75S2	1.000
26	D1+0.45W3+0.75L1+0.75S3	1.000
27	D1+0.45W3+0.75L1+0.75S4	1.000
28	D1+0.45W3+0.75L2+0.75S1	1.000
29	D1+0.45W3+0.75L2+0.75S2	1.000
30	D1+0.45W3+0.75L2+0.75S3	1.000
31	D1+0.45W3+0.75L2+0.75S4	1.000
32	0.6D1+0.6W1	1.000
33	0.6D1+0.6W2	1.000
34	0.6D1+0.6W4	1.000

Member Forces Summary

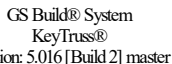
Table indicates: Member ID, max CSI, max axial force, (max comp force if different from max axial force)

TChd	35-1	0.183	591lbs	(-546lbs)	5-6	0.410	-1,952lbs	10-11	0.290	1,068lbs	(-1,059lbs)	15-16	0.366	-2,080lbs	
	1-2	0.186	-1,494lbs		6-7	0.232	-1,507lbs	11-12	0.230	-1,493lbs		16-17	0.172	-1,491lbs	
	2-3	0.371	-2,089lbs		7-8	0.292	1,081lbs	(-1,072lbs)	12-13	0.408	-1,938lbs	17-36	0.183	591lbs	(-546lbs)
	3-4	0.414	-2,587lbs		8-9	0.141	-637lbs		13-14	0.271	-2,095lbs				
	4-5	0.272	-2,109lbs		9-10	0.140	-623lbs		14-15	0.413	-2,573lbs				
BChd	18-19	0.111	-809lbs		22-23	0.302	1,685lbs	(-1,676lbs)	26-27	0.177	-662lbs	30-31	0.317	2,324lbs	(-2,316lbs)
	19-20	0.168	1,295lbs	(-1,272lbs)	23-24	0.282	-1,640lbs		27-28	0.203	-960lbs	31-32	0.328	1,919lbs	(-1,902lbs)
	20-21	0.332	1,943lbs	(-1,925lbs)	24-25	0.205	-980lbs		28-29	0.280	-1,620lbs	32-33	0.172	1,261lbs	(-1,241lbs)
	21-22	0.318	2,345lbs	(-2,336lbs)	25-26	0.179	-682lbs		29-30	0.300	1,664lbs	(-1,656lbs)	33-34	0.129	-706lbs
Webs	1-34	0.256	-920lbs		5-30	0.181	-1,230lbs		10-25	0.072	-416lbs	15-21	0.229	-1,589lbs	
	1-33	0.184	1,386lbs	(-1,272lbs)	5-29	0.207	-1,363lbs		11-25	0.183	-1,079lbs	15-20	0.229	-1,621lbs	
	2-33	0.152	-1,096lbs		6-29	0.059	-390lbs		11-24	0.154	-948lbs	16-20	0.227	-1,589lbs	
	2-32	0.228	-1,599lbs		7-28	0.154	-948lbs		12-23	0.059	-390lbs	16-19	0.154	-1,104lbs	
	3-32	0.230	-1,631lbs		7-27	0.183	-1,078lbs		13-23	0.207	-1,363lbs	17-19	0.173	1,292lbs	(-1,198lbs)
	3-31	0.230	-1,592lbs		8-27	0.072	-416lbs		13-22	0.181	-1,230lbs	17-18	0.264	-892lbs	
	4-31	0.046	-318lbs		9-26	0.044	-232lbs		14-21	0.046	-318lbs				

Additional Notes:

The end of every chord segment shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6" of the end of each chord segment.

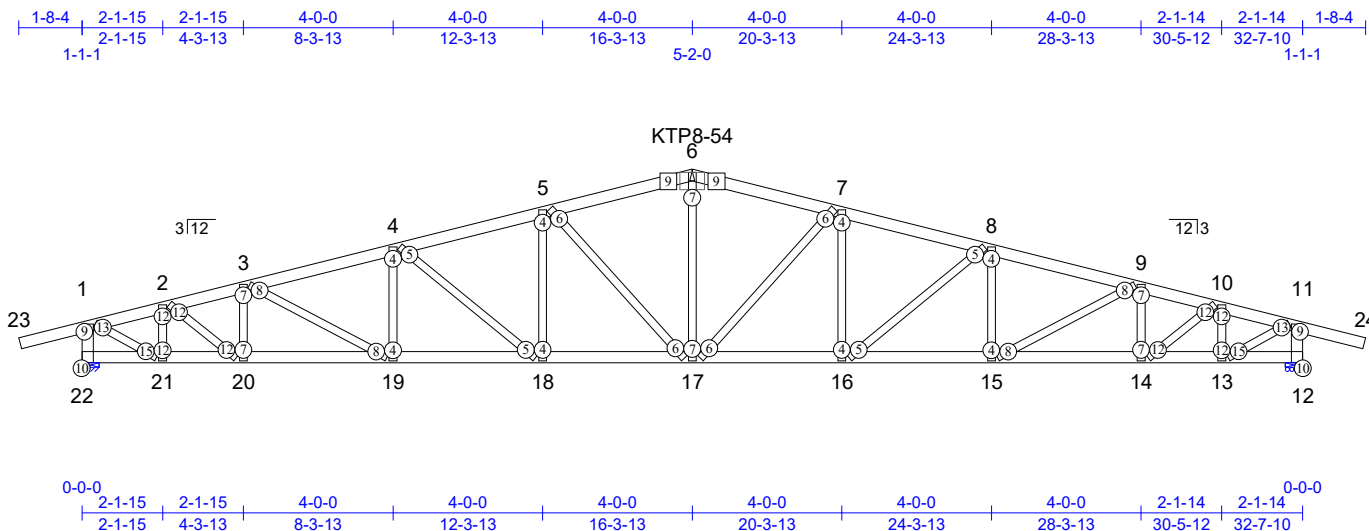
NOTE : Web crippling calculation assume truss is fastened to support.



4001 Hwy 153
Greenville, SC 29611
864 999 2020

Truss: T01-S
JobName: 24188
JobID: Jackson County Airport
Date: 2/27/2025 10:06:49 AM
System: KeyTRUSS 6.113
Page: 1 of 2
Report: Eng Plot

SPAN	PITCH	QTY	OHL	OHR	PLYS	SPACING	WGT/PLY
32-7-10	3/12	2	1-8.4	1-8.4	1	36 in	149.5 lbs



Circles indicate fastener count in webs. Squares indicate fastener count in chords. "Fasteners" indicates the number of #10 SDS (AISI) fasteners required at one end of the member. Each value indicates the number of fasteners required. Where connection plates are called out on this drawing, a plate is required. Refer to General Notes for further clarification. Allowable shear per fastener is calculated per the AISI S100. Maintain fastener spacing at 9/16" min. Maintain fastener edge margin at 9/16" min for each sheet of steel connected.

TC: 0.999(1)
BC: 0.907(13)
Web: 0.955(7-17)

TL: 0.4in
LL: 0.19in
Cant/OHTL: 0.04in
Cant/OHLL: 0.01in
Horz TL: 0.13in

L/9
L/9
2L/9
2L/9

(16-17)
(5-6)
24
24
13

L/240
L/360
2L/180
2L/180

JT	Type	Brg Combo
22	Pin(WL)	1
12	HRoll(WL)	1

JT	Type	Brg Combo	Brg Width	Max React	Grav Uplift	MWFRS Uplift	Max Uplift	Max Horiz
22	Pin (WL)	1	5.5 in	3,332 lbs			-1,313 lbs	628 lbs
12	HRoll (WL)	1	5.5 in	3,332 lbs			-1,313 lbs	

TChd	362S162-54 (50 ksi)
BChd	362S162-43 (50 ksi)
Webs	250S162-33 (50 ksi)
1-22	362S162-68 (50 ksi)

TChd	362S162-54 (50 ksi)
BChd	362S162-43 (50 ksi)
Webs	250S162-33 (50 ksi)
1-22	362S162-68 (50 ksi)

TChdBracing: Sheathed
BChdBracing: 108 in

11-13 250S162-54(50ksi)

11-12 362S162-68 (50 ksi)

1) This truss has been designed in accordance with IBC - 2018.

2) This truss has been designed for the effects due to wind loads in accordance with ASCE7 - 16, V = 110 mph, Exposure C. The building is Gable with Risk Category II, Mean Roof Height 15 ft, Overall Building Dimensions of 67 ft x 131 ft, Enclosure Class: Enclosed, CC Zone Width 6 ft The left end vertical has been exposed to wind. The right end vertical has been exposed to wind. This truss is not an End Zone Truss. This truss is in a Hurricane Proft Region.

3) This truss has been designed for the effects of balanced, minimum and unbalanced snow loads in accordance with ASCE7 - 16, using a ground snow load of 5 psf and terrain category B. The exposure condition is Fully Exposed. The thermal condition is All Others ($C_t = 1.0$) with building category II ($I = 1.0$). The roof ventilation is Unknown with Roof R-Value Unknown. The roof slope is 3/12 and the surface type is All Others. This truss has not been designed to account for the effects of ice dams forming at the eaves.

Load Case Lr1: Std Live Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	Cont		Down	Proj	20psf	20psf	36in
Bot	Cont		Down	Proj	0psf	0psf	36in

Load Case D1: Std Dead Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	Cent		Down	Rake	12 psf	12 psf	36 in
Bot	Cent		Down	Rake	8 psf	8 psf	36 in

User-defined Load Case W10: Drag Left

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	Cont		PLeft	Rake	600 plf	600 plf	
Bot	Cont		PRight	Rake	630 plf	630 plf	

User-defined Load Case W11: Drag Right

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	Cont		PRight	Rate	600 plf	600 plf	
Bot	Cont		PLeft	Rate	630 plf	630 plf	

Load Combinations

#	Load Combo	Factor
1	D1	1.000
2	D1 + Lr1	1.000
3	D1 + S1	1.000
4	D1 + S2	1.000
5	D1 + S3	1.000
6	D1 + S4	1.000
7	D1 + 0.6 W3	1.000
8	D1 + 0.6 W8	1.000

GUSSET PLATES

[illegible]

2/28/2025

ALL GENERAL NOTES OF THIS DRAWING PACKAGE SHALL BE CONSIDERED AS AN INTEGRAL PART OF THIS COMPONENT DESIGN DOCUMENT. NOTE THAT THE PROFESSIONAL ENGINEER'S SEAL INDICATES ONLY THAT THE TRUSS ASSEMBLY SHOWN ON THIS SHEET MEETS THE MINIMUM APPLICABLE DESIGN CRITERIA FOR THE LOADS, LOADING CONDITIONS, TRUSS MEMBER CONFIGURATIONS AND SPANS LISTED ON THIS SHEET.

Keymark Engineering, Inc.
6707 Winchester Circle, Suite 102
Boulder, Colorado 80301

TruTek Framing Systems

4001 Hwy 153
Greenville, SC 29611
864 999 2020

Truss:

JobName: 24188
JobID: Jackson County Airport
Date: 2/27/2025 10:06:50 AM
System: KeyTRUSS 6.113
Page: 2 of 2
Report: Eng Plot

T01-S

SPAN
32-7-10

PITCH
3/12

QTY
2

OHL
1-8-4

OHR
1-8-4

PLYS
1

SPACING
36 in

WGT/PLY
149.5 lbs

9	D1+06W9	1.000
10	D1+06W10	1.000
11	D1+06W11	1.000
12	D1+045W3+0.75Lr1	1.000
13	D1+045W10+0.75Lr1	1.000
14	D1+045W11+0.75Lr1	1.000
15	D1+045W3+0.75Sl	1.000
16	D1+045W3+0.75S2	1.000
17	D1+045W3+0.75S3	1.000
18	D1+045W3+0.75S4	1.000
19	D1+045W10+0.75Sl	1.000
20	D1+045W10+0.75S2	1.000
21	D1+045W10+0.75S3	1.000
22	D1+045W10+0.75S4	1.000
23	D1+045W11+0.75Sl	1.000
24	D1+045W11+0.75S2	1.000
25	D1+045W11+0.75S3	1.000
26	D1+045W11+0.75S4	1.000
27	06D1+06W1	1.000
28	06D1+06W2	1.000
29	06D1+06W4	1.000
30	06D1+06W10	1.000
31	06D1+06W11	1.000

Member Forces Summary

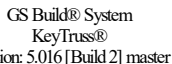
Table indicates: Member ID,max CSL,max axial force,(max comp force if different from max axial force)

TChd	23-1	0.158	699lbs	(-672lbs)	3-4	0.947	-6,334lbs	6-7	0.597	-4,034lbs	9-10	0.758	-5,565lbs		
	1-2	0.899	-3,969lbs		4-5	0.755	-5,382lbs	7-8	0.757	-5,381lbs	10-11	0.898	-3,961lbs		
	2-3	0.754	-5,572lbs		5-6	0.599	-4,034lbs	8-9	0.951	-6,332lbs	11-24	0.158	699lbs		
BChd	12-13	0.247	748lbs	(-748lbs)	15-16	0.581	5,431lbs	(-1,151lbs)	18-19	0.535	4,962lbs	(-525lbs)	21-22	0.351	-1,376lbs
	13-14	0.692	3,887lbs	(-1,329lbs)	16-17	0.454	4,453lbs	(-327lbs)	19-20	0.614	4,824lbs	(-961lbs)			
	14-15	0.664	5,289lbs	(-1,581lbs)	17-18	0.408	3,984lbs	(0lbs)	20-21	0.642	3,425lbs	(-714lbs)			
Webs	1-22	0.430	-3,374lbs		4-19	0.255	630lbs	(-622lbs)	7-16	0.188	720lbs	(-356lbs)	10-13	0.887	-2,599lbs
	1-21	0.642	5,149lbs	(-2,464lbs)	4-18	0.679	-975lbs		8-16	0.679	-975lbs		11-13	0.640	5,139lbs
	2-21	0.888	-2,601lbs		5-18	0.187	718lbs	(-354lbs)	8-15	0.256	632lbs	(-623lbs)	11-12	0.430	-3,370lbs
	2-20	0.531	2,568lbs	(-1,358lbs)	5-17	0.953	-1,108lbs		9-15	0.729	1,603lbs	(-1,222lbs)			
	3-20	0.480	-1,342lbs		6-17	0.275	1,332lbs	(0lbs)	9-14	0.481	-1,346lbs				
	3-19	0.724	1,596lbs	(-1,215lbs)	7-17	0.955	-1,110lbs		10-14	0.532	2,571lbs	(-1,363lbs)			

Additional Notes:

The end of every chord segment shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6 " of the end of each chord segment.

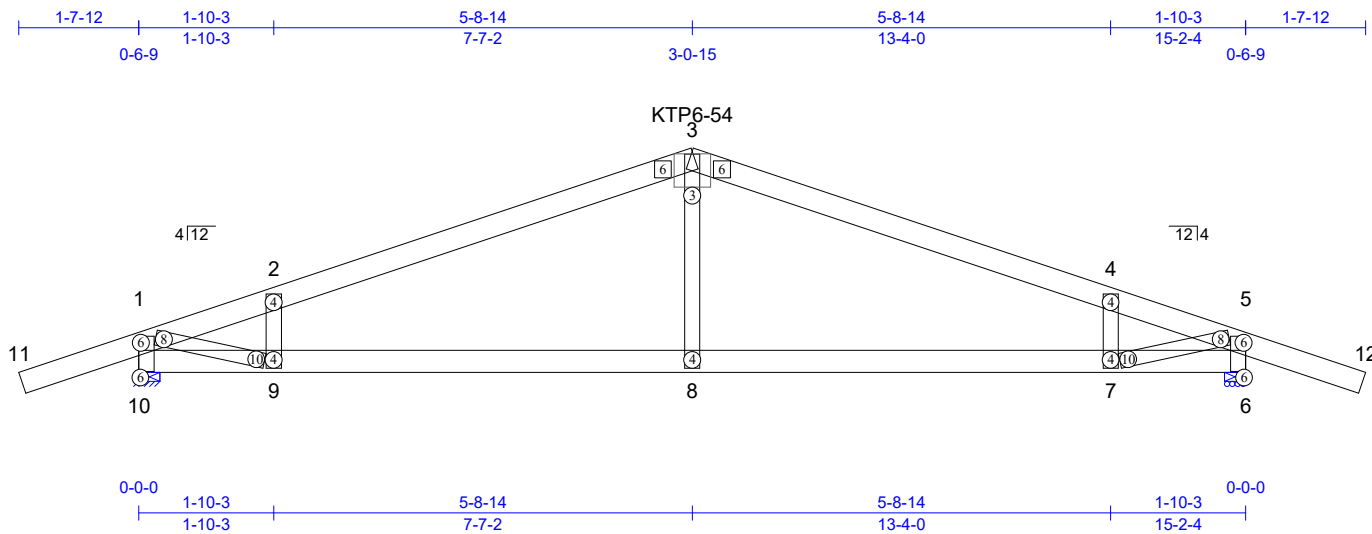
NOTE : Web crippling calculation assume truss is fastened to support.



4001 Hwy 153
Greenville, SC 29611
864 999 2020

Truss: T02
JobName: 24188
JobID: Jackson County Airport
Date: 2/27/2025 10:06:53 AM
System: KeyTRUSS 6.113
Page: 1 of 1
Report: Eng Plot

SPAN	PITCH	QTY	OHL	OHR	PLYS	SPACING	WGT/PLY
15-24	4/12	6	1-7-12	1-7-12	1	48 in	45 lbs



Circles indicate fastener count in webs. Squares indicate fastener count in chords. "Fasteners" indicates the number of #10 SDS (AISI) fasteners required at one end of the member. Each value indicates the number of fasteners required. Where connection plates are called out on this drawing, a plate is required. Refer to General Notes for further clarification. Allowable shear per fastener is calculated per the AISI S100. Maintain fastener spacing at 9/16" min. Maintain fastener edge margin at 9/16" min for each sheet of steel connected.

TC: 1.004(3-4)
BC: 0.640(7-8)
Web: 0.408(5-6)

TL: 0.26in
LL: 0.13in
Cant / OHTL: 0.04in
Cant / OHLL: 0.02in
Hrz TL : 0.05in

L/ 685
L/ 999
2L/ 946
2L/ 999

(2-3)
(2-3)
12
12
2

L/240
L/360
2L/180
2L/180

JT	Type	BrgCombo	BrgWidth	MaxReact	GravUplift	MWPRSUplift	MaxUplift	MaxHoriz
10	Pin(WL)	1	35in	1,450lbs				80lbs
6	HRoll(WL)	1	35in	1,450lbs				

TChd	362SI62-43 (50 ksi)
BChd	362SI62-33 (50 ksi)
Webs	250SI62-33 (50 ksi)

TChdBracing: Sheathed
BChdBracing: 108 in

- 1) This truss has been designed in accordance with IBC - 2018.
- 2) This truss has been designed for the effects due to standard loading of TC Live = 20 psf; TC Dead = 12 psf; BC Live = 0 psf; BC Dead = 8 psf at 48 ioc.
- 3) This truss has been designed for the effects due to wind loads in accordance with ASCE7 - 16, V = 110 mph, Exposure C. The building is Gable with Risk Category II, Mean Roof Height 15 ft, Overall Building Dimensions of 67 ft x 131 ft, Enclosure Class: Enclosed, CC Zone: Width 6 ft The left end vertical has been exposed to wind. The right end vertical has been exposed to wind. This truss is not an End Zone Truss. This truss is in a Hurricane Prone Region.
- 4) This truss has been designed for the effects of balanced and unbalanced snow loads in accordance with ASCE7 - 16, using a ground snow load of 5 psf and terrain category B. The exposure condition is Fully Exposed. The thermal condition is All Others (Ct = 1.0) with building category II (I = 1.0). The roof ventilation is Unknown with Roof R-Value Unknown. The roof slope is 4/12 and the surface type is All Others. This truss has not been designed to account for the effects of ice dams forming at the eaves.

Table indicates: Member ID, max CSL, max axial force, (max compr force if different from max axial force)

TChd	11-1	0.248	721lbs	(01bs)	2-3	1.004	-1,908lbs	4-5	0.349	-1,814lbs	
	1-2	0.349	-1,814lbs		3-4	1.004	-1,908lbs	5-12	0.248	721lbs	(01bs)
BChd	6-7	0.392	161bs	(-111bs)	7-8	0.640	1,693lbs	(-2281bs)	8-9	0.640	1,693lbs
Wchs	1-10	0.408	-1,267lbs		2-9	0.165	-505lbs		4-7	0.165	-505lbs
	1-9	0.358	1,732lbs	(-2991bs)	3-8	0.084	404lbs	(01bs)	5-7	0.358	1,732lbs
										(-2501bs)	
									5-6	0.408	-1,267lbs

Additional Notes:

The end of every chord segment shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6" of the end of each chord segment.

NOTE : Web crippling calculation assume truss is fastened to support.

GUSSET PLATES

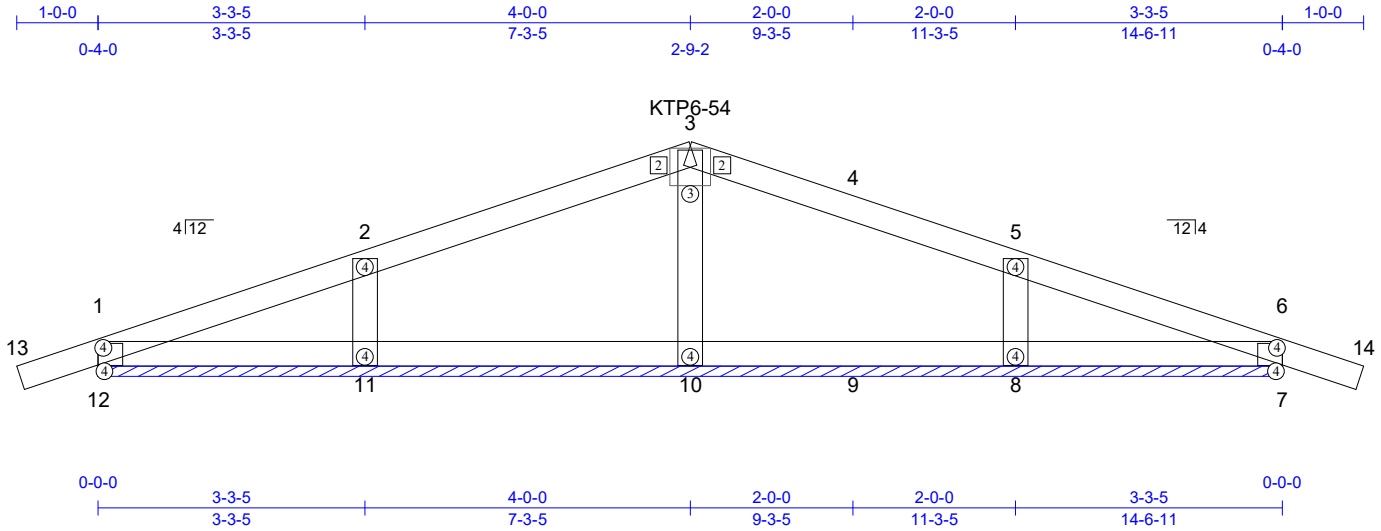
[illegible]

2/28/2025

ALL GENERAL NOTES OF THIS DRAWING PACKAGE SHALL BE CONSIDERED AS AN INTEGRAL PART OF THIS COMPONENT DESIGN DOCUMENT. NOTE THAT THE PROFESSIONAL ENGINEER'S SEAL INDICATES ONLY THAT THE TRUSS ASSEMBLY SHOWN ON THIS SHEET MEETS THE MINIMUM APPLICABLE DESIGN CRITERIA FOR THE LOADS, LOADING CONDITIONS, TRUSS MEMBER CONFIGURATIONS AND SPANS LISTED ON THIS SHEET.

Keymark Engineering, Inc.
6707 Winchester Circle, Suite 102
Boulder, Colorado 80301

SPAN	PITCH	QTY	OHL	OHR	PLYS	SPACING	WGT/PLY
14-6-11	4/12	1	1-0-0	1-0-0	1	48 in	35.2 lbs



Circles indicate fastener count in webs. Squares indicate fastener count in chords. "Fasteners" indicates the number of #10 SDS (AISI) fasteners required at one end of the member. Each value indicates the number of fasteners required. Where connection plates are called out on this drawing, a plate is required. Refer to General Notes for further clarification. Allowable shear per fastener is calculated per the AISI S100. Maintain fastener spacing at 9/16" min. Maintain fastener edge margin at 9/16" min for each sheet of steel connected.

CSI Summary

TC : 0.405 (2-3)
BC : 0.093 (8-10)
Web: 0.181 (2-11)

Deflection

TL: 0.03 in
LL: 0.02 in
Cant / OHTL: 0 in
Cant / OHLL: 0 in
Hrz TL: 0 in

L/

L / 999
L / 999
2L / 999
2L / 999

(loc)

(2-3)
(2-3)
13
13
14

Allowed

L / 240
L / 360
2L / 180
2L / 180

Reaction Summary

JT	Type	Brg Combo	Brg Width	Max React	Grav Uplift	MWFR Uplift	Max Uplift	Max Horiz
7	HRoll (WL)	1	174.688 in	369 lbs				
8	HRoll (WL)	1	174.688 in	674 lbs				
10	HRoll (WL)	1	174.688 in	543 lbs				
11	HRoll (WL)	1	174.688 in	674 lbs				
12	Pn (WL)	1	174.688 in	369 lbs		-3 lbs	-3 lbs	-66 lbs

Material Summary

TCld 362SI 62-33 (50 ksi)
BCld 362SI 62-33 (50 ksi)
Webs 362SI 62-33 (50 ksi)
TCld Bacing: 48 in
BCld Bacing: 108 in

Loads Summary

- This truss has been designed in accordance with IBC - 2018.
- This truss has been designed for the effects due to standard loading of TC Live = 20 psf; TC Dead = 12 psf; BC Live = 0 psf; BC Dead = 8 psf at 48 ioc.
- This truss has been designed for the effects due to wind loads in accordance with ASCE7 - 16, V = 110 mph, Exposure C. The building is Gable with Risk Category II, Mean Roof Height 15 ft, Overall Building Dimensions of 67 ft x 131 ft, Enclosure Class: Enclosed, CC Zone Width 6 ft The left end vertical has been exposed to wind. The right end vertical has been exposed to wind. This truss is not an End Zone Truss. This truss is in a Hurricane Prone Region.
- This truss has been designed for the effects of balanced and unbalanced snow loads in accordance with ASCE7 - 16, using a ground snow load of 5 psf and terrain category B. The exposure condition is Fully Exposed. The thermal condition is All Others (Ct = 1.0) with building category II (I = 1.0). The roof ventilation is Unknown with Roof R-Value Unknown. The roof slope is 4/12 and the surface type is All Others. This truss has not been designed to account for the effects of ice dams forming at the eaves.

Member Forces Summary

Table indicates: Member ID, max CSI, max axial force, (max compr force if different from max axial force)

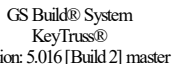
TCld	13-1	0.181	481 lbs	(0 lbs)	2-3	0.405	159 lbs	(-118 lbs)	5-6	0.369	-71 lbs	(0 lbs)
1-2	0.369	-71 lbs			3-5	0.405	156 lbs	(-118 lbs)	6-14	0.181	481 lbs	
BCld	7-8	0.081	94 lbs	(-20 lbs)	8-10	0.093	94 lbs	(-20 lbs)	10-11	0.093	94 lbs	(-20 lbs)
11-12	0.081	94 lbs	(-20 lbs)		11-12	0.081	94 lbs	(-20 lbs)				
Webs	1-12	0.134	-318 lbs		3-10	0.144	-410 lbs		6-7	0.134	-318 lbs	
2-11	0.181	-559 lbs			5-8	0.181	-559 lbs					

Additional Notes:

The end of every chord segment shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6" of the end of each chord segment.
NOTE: Web crippling calculation assume truss is fastened to support.



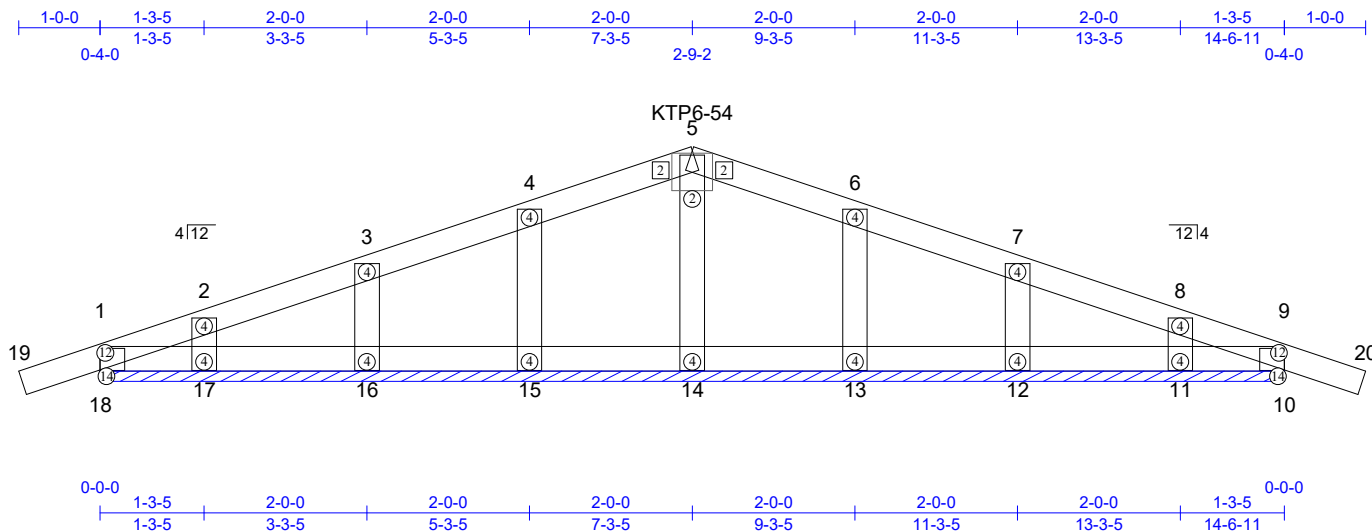
2/28/2025



4001 Hwy 153
Greenville, SC 29611
864 999 2020

Truss: T02-E
JobName: 24188
JobID: Jackson County Airport
Date: 2/27/2025 10:06:52 AM
System: KeyTRUSS 6.113
Page: 1 of 2
Report: Eng Plot

SPAN	PITCH	QTY	OHL	OHR	PLYS	SPACING	WGT/PLY
14'-6"-11"	4/12	1	1'-0"-0"	1'-0"-0"	1	48" in	40.6 lbs



Circles indicate fastener count in webs. Squares indicate fastener count in chords. "Fasteners" indicates the number of #10 SDS (AISI) fasteners required at one end of the member. Each value indicates the number of fasteners required. Where connection plates are called out on this drawing, a plate is required. Refer to General Notes for further clarification. Allowable shear per fastener is calculated per the AISI S100. Maintain fastener spacing at 9/16" min. Maintain fastener edge margin at 9/16" min for each sheet of steel connected.

TC: 0.637(8-9)
BC: 0.665(17-18)
Web: 0.878(9-10)

TL: 0in
LL: 0in
Cant/OHTL: 0.01 in
Cant/OHLL: 0inUP
Hrztl: 0.01 in

L/
L/ 999
L/ 999
2L/ 999
2L/ 999

(loc)
(4-5)
(4-5)
19
19
5

L/240
L/360
2L/180
2L/180

JT	Type	Brg Combo	Brg Width	Max React	Grav Uplift	MWFRS Uplift	Max Uplift	Max Horiz
10	HRoll (WL)	1	174.688 in	1,138 lbs	-887 lbs	-668 lbs	-887 lbs	
11	HRoll (WL)	1	174.688 in	332 lbs	-92 lbs	-271 lbs	-92 lbs	
12	HRoll (WL)	1	174.688 in	337 lbs				
13	HRoll (WL)	1	174.688 in	353 lbs				
14	HRoll (WL)	1	174.688 in	271 lbs				
15	HRoll (WL)	1	174.688 in	353 lbs				
16	HRoll (WL)	1	174.688 in	337 lbs				
17	HRoll (WL)	1	174.688 in	331 lbs	-92 lbs	-271 lbs	-92 lbs	
18	Pin (WL)	1	174.688 in	1,136 lbs	-885 lbs	-666 lbs	-885 lbs	-66 lbs

[illegible]

TChd	362SI62-33 (50 ksi)	
BChd	362SI62-33 (50 ksi)	
Welds	362SI62-33 (50 ksi)	except:
1-18	362SI62-43 (50 ksi)	9-10 362SI62-43 (50 ksi)

- 1) This truss has been designed in accordance with IBC - 2018.
- 2) This truss has been designed for the effects due to wind loads in accordance with ASCE7 - 16, V = 110 mph, Exposure C. The building is Gable with Risk Category II, Mean Roof Height 15 ft, Overall Building Dimensions of 67 ft x 131 ft, Enclosure Class: Enclosed, CC Zone Width 6 ft. The left end vertical has been exposed to wind. The right end vertical has been exposed to wind. This truss is not an End Zone Truss. This truss is in a Hurricane Prone Region.
- 3) This truss has been designed for the effects of balanced and unbalanced snow loads in accordance with ASCE7 - 16, using a ground snow load of 5 psf and terrain category B. The exposure condition is Fully Exposed. The thermal condition is All Others (Ct = 1.0) with building category II (I = 1.0). The roof ventilation is Unknown with Roof R-Value Unknown. The roof slope is 4/12 and the surface type is All Others. This truss has not been designed to account for the effects of ice dams forming at the eaves.

Load Case Lr1: Std Live Load

Distributed Loads							
Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	Cent		Down	Proj	20psf	20psf	48in
Bot	Cent		Down	Proj	0psf	0psf	48in

Load Case D1: Std Dead Load

Distributed Loads							
Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	Cont		Down	Rake	12 psf	12 psf	48 in
Bot	Cont		Down	Rake	8 psf	8 psf	48 in

User-defined Load Case L1: Drag Right

Distributed Loads							
Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	Cont		PRight	Rake	300 plf	300 plf	
Bot	Cont		PLeft	Rake	342 plf	342 plf	

User-defined Load Case L2: Drag Left

Distributed Loads							
Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	Cent		PLeft	Rake	300 plf	300 plf	
Bot	Cent		PRight	Rake	342 plf	342 plf	

Load Combinations

#	Load Combo	Factor
1	D1	1.000
2	D1+L1	1.000

ALL GENERAL NOTES OF THIS DRAWING PACKAGE SHALL BE CONSIDERED AS AN INTEGRAL PART OF THIS COMPONENT DESIGN DOCUMENT. NOTE THAT THE PROFESSIONAL ENGINEER'S SEAL INDICATES ONLY THAT THE TRUSS ASSEMBLY SHOWN ON THIS SHEET MEETS THE MINIMUM APPLICABLE DESIGN CRITERIA FOR THE LOADS, LOADING CONDITIONS, TRUSS MEMBER CONFIGURATIONS AND SPANS LISTED ON THIS SHEET.

Keymark Engineering, Inc.
6707 Winchester Circle, Suite 102
Boulder, Colorado 80301

2/28/2025

SPAN	PITCH	QTY	OHL	OHR	PLYS	SPACING	WGT/PLY
14-6-11	4/12	1	1-0-0	1-0-0	1	48 in	40.6 lbs
3	D1+L2			1.000			
4	D1+Lr1			1.000			
5	D1+S1			1.000			
6	D1+S2			1.000			
7	D1+S3			1.000			
8	D1+0.75L1+0.75Lr1			1.000			
9	D1+0.75L2+0.75Lr1			1.000			
10	D1+0.75L1+0.75S1			1.000			
11	D1+0.75L1+0.75S2			1.000			
12	D1+0.75L1+0.75S3			1.000			
13	D1+0.75L2+0.75S1			1.000			
14	D1+0.75L2+0.75S2			1.000			
15	D1+0.75L2+0.75S3			1.000			
16	D1+0.6W3			1.000			
17	D1+0.6W8			1.000			
18	D1+0.6W9			1.000			
19	D1+0.45W3+0.75L1+0.75Lr1			1.000			
20	D1+0.45W3+0.75L2+0.75Lr1			1.000			
21	D1+0.45W3+0.75L1+0.75S1			1.000			
22	D1+0.45W3+0.75L1+0.75S2			1.000			
23	D1+0.45W3+0.75L1+0.75S3			1.000			
24	D1+0.45W3+0.75L2+0.75S1			1.000			
25	D1+0.45W3+0.75L2+0.75S2			1.000			
26	D1+0.45W3+0.75L2+0.75S3			1.000			
27	0.6D1+0.6W1			1.000			
28	0.6D1+0.6W2			1.000			
29	0.6D1+0.6W4			1.000			

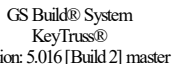
Member Forces Summary

Table indicates: Member ID,max CSL,max axial force,(max compr force if different from max axial force)

TChd	19-1	0.181	382lbs	(-346lbs)	3-4	0.335	-1,280lbs	6-7	0.336	-1,285lbs	9-20	0.181	382lbs	(-346lbs)		
	1-2	0.636	-2,288lbs	4-5	0.184	-651lbs	7-8	0.464	-1,915lbs							
	2-3	0.463	-1,910lbs	5-6	0.185	-656lbs	8-9	0.637	-2,293lbs							
BCld	10-11	0.664	2,436lbs	(-2,433lbs)	12-13	0.335	1,367lbs	(-1,363lbs)	14-15	0.178	689lbs	(-685lbs)	16-17	0.518	2,057lbs	(-2,053lbs)
	11-12	0.516	2,051lbs	(-2,047lbs)	13-14	0.176	683lbs	(-679lbs)	15-16	0.336	1,373lbs	(-1,369lbs)				
Webs	1-18	0.877	-996lbs		4-15	0.097	-291lbs		7-12	0.086						
	2-17	0.068	-214lbs		5-14	0.072	-206lbs		8-11	0.068						
	3-16	0.086	-267lbs		6-13	0.097	-291lbs		9-10	0.878	-997lbs					

Additional Notes:

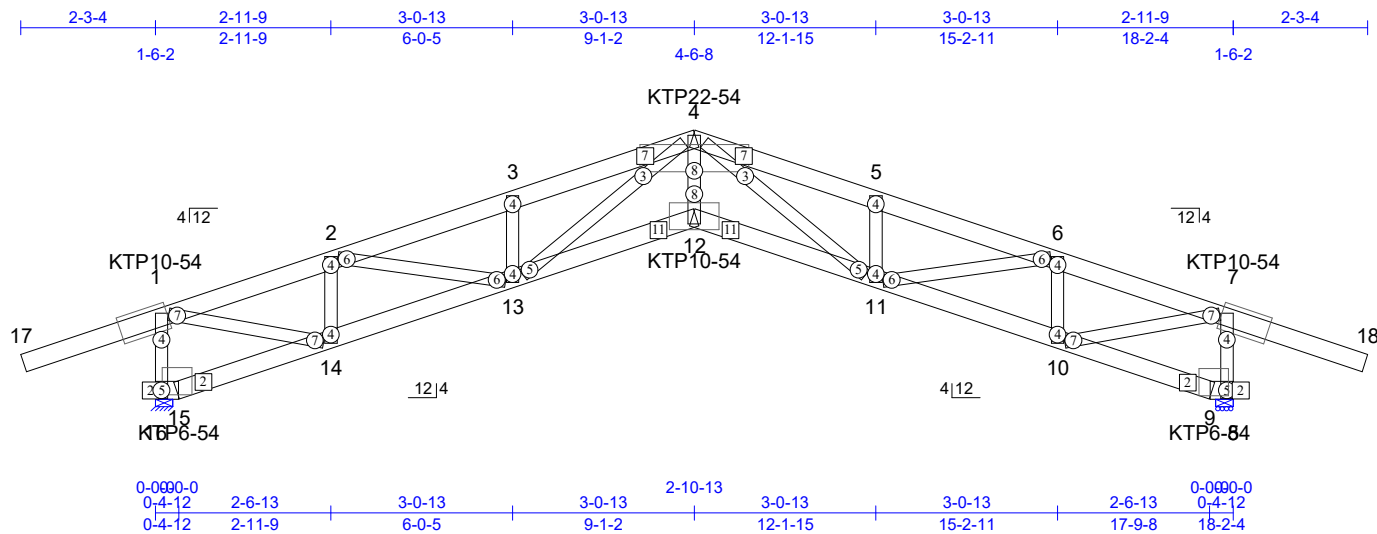
The end of every chord segment shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6 " of the end of each chord segment.
NOTE : Web crippling calculation assume truss is fastened to support.



4001 Hwy 153
Greenville, SC 29611
864 999 2020

Truss: T03
JobName: 24188
JobID: Jackson County Airport
Date: 5/14/2025 10:44:07 AM
System: KeyTRUSS 6.113
Page: 1 of 1
Report: Eng Plot

SPAN	PITCH	QTY	OHL	OHR	PLYS	SPACING	WGT/PLY
18-24	4/12	7	2-3/4	2-3/4	1	48 in	80.5 lbs



Circles indicate fastener count in webs. Squares indicate fastener count in chords. "Fasteners" indicates the number of #10 SDS (AISI) fasteners required at one end of the member. Each value indicates the number of fasteners required. Where connection plates are called out on this drawing, a plate is required. Refer to General Notes for further clarification. Allowable shear per fastener is calculated per the AISI S100. Maintain fastener spacing at 9/16" min. Maintain fastener edge margin at 9/16" min for each sheet of steel connected.

TC : 0922(5-6)
BC : 0801 (12-13)
Web: 0693(4-12)

TL: 0.49in
LL: 0.23in
Cant / OHTL: 0.12inUP
Cant / OHLL: 0.04inUP
Horz TL: 0.36in

L/434
L/913
2L/441
2L/999

12
12
18
17
8

L/240
L/360
2L/180
2L/180

JT	Type	Brg Combo
16	Pin(WL)	1
8	HRoll(WL)	1

TChd	362SI62-43 (50 ksi)
BChd	362SI62-33 (50 ksi)
Webs	250SI62-33 (50 ksi)
1-16	250SI62-43 (50 ksi)

except:

250S162-43 (50ksi)

TChdBracing: Sheathed
BChdBracing: 108 in

1) This truss has been designed in accordance with IBC - 2018.

2) This truss has been designed for the effects due to standard loading of TC Live = 20 psf; TC Dead = 12 psf; BC Live = 0 psf; BC Dead = 8 psf at 48 joe.

3) This truss has been designed for the effects due to wind loads in accordance with ASCE7 - 16, V = 110 mph, Exposure C. The building is Gable with Risk Category II, Mean Roof Height 20 ft, Overall Building Dimensions of 67 ft x 131 ft, Enclosure Class: Enclosed, CC Zone Width 6.7 ft The left end vertical has been exposed to wind. The right end vertical has been exposed to wind. This truss is not an End Zone Truss. This truss is in a Hurricane Prone Region.

4) This truss has been designed for the effects of balanced and unbalanced snow loads in accordance with ASCE7 - 16, using a ground snow load of 5 psf and terrain category B. The exposure condition is Fully Exposed. The thermal condition is All Others (Ct = 1.0) with building category II (I = 1.0). The roof ventilation is Unknown with Roof R-Value Unknown. The roof slope is 4/12 and the surface type is All Others. This truss has not been designed to account for the effects of ice dams forming at the eaves.

Table indicates: Member ID, max CSI, max axial force, (max comp. force if different from max axial force)

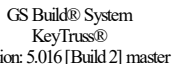
TChd	17-1	0.440	981lbs	(01lbs)	2-3	0.922	-4,360lbs	4-5	0.856	-4,366lbs	6-7	0.596	-2,596lbs	(01lbs)		
	1-2	0.596	-2,596lbs		2-4	0.856	-4,360lbs	5-6	0.922	-4,360lbs	7-8	0.440	981lbs			
BChd	8-9	0.007	47lbs	(-29lbs)	10-11	0.390	2,546lbs	(-249lbs)	12-13	0.504	5,024lbs	(-346lbs)	14-15	0.345	1241lbs	(-123lbs)
	9-10	0.217	61lbs	(-26lbs)	11-12	0.801	5,024lbs	(-345lbs)	13-14	0.801	2,546lbs	(-65lbs)	15-16	0.029	-131lbs	
Webbs	1-16	0.433	-1,782lbs		3-13	0.135	-401lbs		5-11	0.135	-401lbs		7-8	0.433	-1,782lbs	
	1-14	0.503	2,433lbs	(-143lbs)	4-13	0.405	-840lbs		6-11	0.353	1,709lbs	(-257lbs)				
	2-14	0.375	-1,108lbs		4-12	0.693	3,352lbs	(-180lbs)	6-10	0.375	-1,108lbs					
	2-13	0.353	1,709lbs	(-304lbs)	4-11	0.405	-840lbs		7-10	0.503	2,433lbs	(-277lbs)				

Additional Notes:

The end of every chord segment shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6" of the end of each chord segment.
NOTE: Web crippling calculation assume truss is fastened to support.

[illegible]

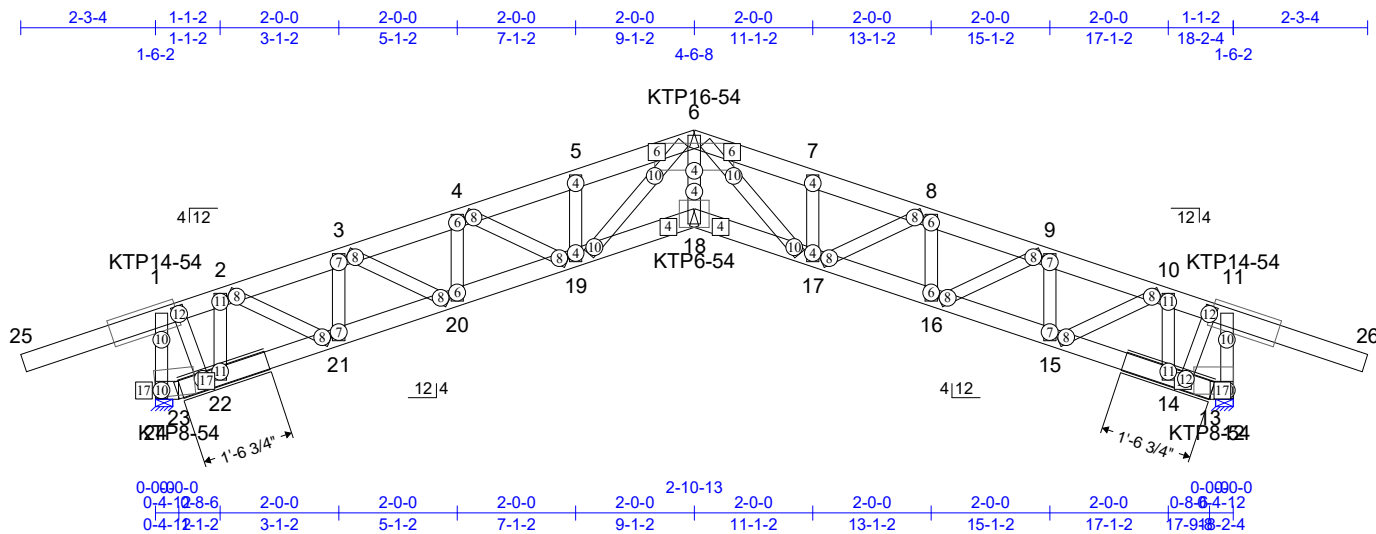
5/14/2025



4001 Hwy 153
Greenville, SC 29611
864 999 2020

Truss: T03-S1
JobName: 24188
JobID: Jackson County Airport
Date: 5/14/2025 8:39:17 AM
System: KeyTRUSS 6.113
Page: 1 of 2
Report: Eng Plot

SPAN	PITCH	QTY	OHL	OHR	PLYS	SPACING	WGT/PLY
18-24	4/12	1	2-3-4	2-3-4	1	52 in	127.2 lbs



Circles indicate fastener count in webs. Squares indicate fastener count in chords. "Fasteners" indicates the number of #10 SDS (AISI) fasteners required at one end of the member. Each value indicates the number of fasteners required. Where connection plates are called out on this drawing, a plate is required. Refer to General Notes for further clarification. Allowable shear per fastener is calculated per the AISI S100. Maintain fastener spacing at 9/16" min. Maintain fastener edge margin at 9/16" min for each sheet of steel connected.

TC: 0.959(11)
BC: 0.695(22-23)
Web: 0.820(10-14)

TL: 0.06in
LL: 0.02in
Cant / OHTL: 0.03in
Cant / OHLL: 0.02in
Horz TL: 0.04in

L/999
L/999
2L/999
2L/999

(19-20)
(19-20)
25
26
25

L/240
L/360
2L/180
2L/180

JT	Type	Brg Combo
24	PIn(WL)	1
12	PIn(WL)	1

TChd	362S162-68 (50 ksi)
BChd	362S162-68 (50 ksi)
Webs	250S162-33 (50 ksi)

1-24	250S162-54(50ksi)	1-22	250S162-54(50ksi)	11-14	250S162-54(50ksi)	11-12	250S162-54(50ksi)
<p>Bold lines indicate track reinforcement is required on the hard side of the cee. Track shall match gauge/depth of cee material.</p> <p>Track shall be attached with fasteners through track into cee using:</p> <p>1 row 12" o.c. through each flange minimum 3 fasteners per row</p> <p>1 row 12" o.c. through the web minimum 3 fasteners per row</p> <p>At truss joints, fasteners that connect webs to chords may be counted as track reinforcement fasteners.</p>							

[illegible]

1) This truss has been de

- 1) This truss has been designed in accordance with IBC - 2018.
- 2) This truss has been designed for the effects due to wind loads in accordance with ASCE7 - 16, V = 110 mph, Exposure C. The building is Gable with Risk Category II, Mean Roof Height 20 ft, Overall Building Dimensions of 67 ft x 131 ft, Enclosure Class: Enclosed, CC Zone Width 6.7 ft The left end vertical has been exposed to wind. The right end vertical has been exposed to wind. This truss is not an End Zone Truss. This truss is in a Hurricane Prone Region.
- 3) This truss has been designed for the effects of balanced and unbalanced snow loads in accordance with ASCE7 - 16, using a ground snow load of 5 psf and terrain category B. The exposure condition is Fully Exposed. The thermal condition is All Others (Ct = 1.0) with building category II (I = 1.0). The roof ventilation is Unknown with Roof R-Value Unknown. The roof slope is 4/12 and the surface type is All Others. This truss has not been designed to account for the effects of ice dams forming at the eaves.

Load Case Lr1: Std Live Load

Distributed Loads							
Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	Cont		Down	Proj	20psf	20psf	52in
Bot	Cont		Down	Proj	0psf	0psf	52in

Load Case D1: Std Dead Load

Distributed Loads							
Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	Cont		Down	Rake	12psf	12psf	52in
Bot	Cont		Down	Rake	8psf	8psf	52in

User-defined Load Case W10: Drag Right

Distributed Loads						
Member	Location 1	Location 2	Direction	Spread	Start Load	End Load
Top	Cent		PRight	Rake	900 plf	900 plf

User-defined Load Case W11: Drag Left

Distributed Loads						
Member	Location 1	Location 2	Direction	Spread	Start Load	End Load
Top	Cent		PLeft	400	900 plf	900 plf

Load Combinations

#	Load/Combo	Factor
1	D1	1.000
2	D1+Lr1	1.000
3	D1+S1	1.000
4	D1+S2	1.000
5	D1+S3	1.000
6	D1+0.6W3	1.000



5/14/2025

ALL GENERAL NOTES OF THIS DRAWING PACKAGE SHALL BE CONSIDERED AS AN INTEGRAL PART OF THIS COMPONENT DESIGN DOCUMENT. NOTE THAT THE PROFESSIONAL ENGINEER'S SEAL INDICATES ONLY THAT THE TRUSS ASSEMBLY SHOWN ON THIS SHEET MEETS THE MINIMUM APPLICABLE DESIGN CRITERIA FOR THE LOADS, LOADING CONDITIONS, TRUSS MEMBER CONFIGURATIONS AND SPANS LISTED ON THIS SHEET.

Keymark Engineering, Inc.
6707 Winchester Circle, Suite 102
Boulder, Colorado 80301

TruTek Framing Systems

4001 Hwy 153
Greenville, SC 29611
864 999 2020

Truss: T03-S1
JobName: 24188
JobID: Jackson County Airport
Date: 5/14/2025 8:39:17 AM
System: KeyTRUSS 6.113
Page: 2 of 2
Report: Eng Plot

SPAN	PITCH	QTY	OHL	OHR	PLYS	SPACING	WGT/PLY
18-24	4/12	1	2-3-4	2-3-4	1	52 in	127.2 lbs
7	D1+06W8			1000			
8	D1+06W9			1000			
9	D1+06W10			1000			
10	D1+06W11			1000			
11	D1+045W3+075Lr1			1000			
12	D1+045W10+075Lr1			1000			
13	D1+045W11+075Lr1			1000			
14	D1+045W3+075Sl			1000			
15	D1+045W3+075S2			1000			
16	D1+045W3+075S3			1000			
17	D1+045W10+075Sl			1000			
18	D1+045W10+075S2			1000			
19	D1+045W10+075S3			1000			
20	D1+045W11+075Sl			1000			
21	D1+045W11+075S2			1000			
22	D1+045W11+075S3			1000			
23	06D1+06W1			1000			
24	06D1+06W2			1000			
25	06D1+06W4			1000			
26	06D1+06W10			1000			
27	06D1+06W11			1000			

Member Forces Summary

Table indicates: Member ID,max CSL,max axial force,(max comp force if different from max axial force)

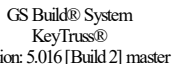
TChd	25-1	0273	1,393 lbs	(-1,327 lbs)	3-4	0.198	-2,107 lbs	6-7	0.306	-2,156 lbs	9-10	0.201	-1,662 lbs
	1-2	0.599	-1,159 lbs		4-5	0.189	-2,064 lbs	7-8	0.189	-2,067 lbs	10-11	0.599	-1,159 lbs
	2-3	0.201	-1,663 lbs		5-6	0.306	-2,159 lbs	8-9	0.198	-2,109 lbs	11-26	0.273	1,393 lbs
BChd	12-13	0.546	-7,159 lbs		15-16	0.442	-4,280 lbs	18-19	0.141	-1,200 lbs	21-22	0.621	-5,895 lbs
	13-14	0.694	-6,791 lbs		16-17	0.353	-2,742 lbs	19-20	0.353	-2,747 lbs	22-23	0.695	-6,793 lbs
	14-15	0.624	-5,892 lbs		17-18	0.140	-1,200 lbs	20-21	0.442	-4,283 lbs	23-24	0.546	-7,162 lbs
Webs	1-24	0.682	-3,851 lbs		4-20	0.372	-1,100 lbs	7-17	0.096	-286 lbs	10-14	0.820	-2,403 lbs
	1-22	0.595	4,735 lbs	(-3,360 lbs)	4-19	0.592	-1,658 lbs	8-17	0.592	-1,659 lbs	11-14	0.595	4,737 lbs
	2-22	0.818	-2,400 lbs		5-19	0.096	-286 lbs	8-16	0.373	-1,101 lbs	11-12	0.682	-3,851 lbs
	2-21	0.340	1,644 lbs	(-851 lbs)	6-19	0.580	2,285 lbs	9-16	0.462	1,616 lbs			
	3-21	0.465	-1,372 lbs		6-18	0.228	-675 lbs	9-15	0.465	-1,372 lbs			
	3-20	0.461	1,614 lbs	(-1,297 lbs)	6-17	0.581	2,286 lbs	(-1,496 lbs)	10-15	0.340	1,644 lbs	(-850 lbs)	

Additional Notes:

The end of every chord segment shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6 " of the end of each chord segment.

NOTE : Web crippling calculation assume truss is fastened to support.

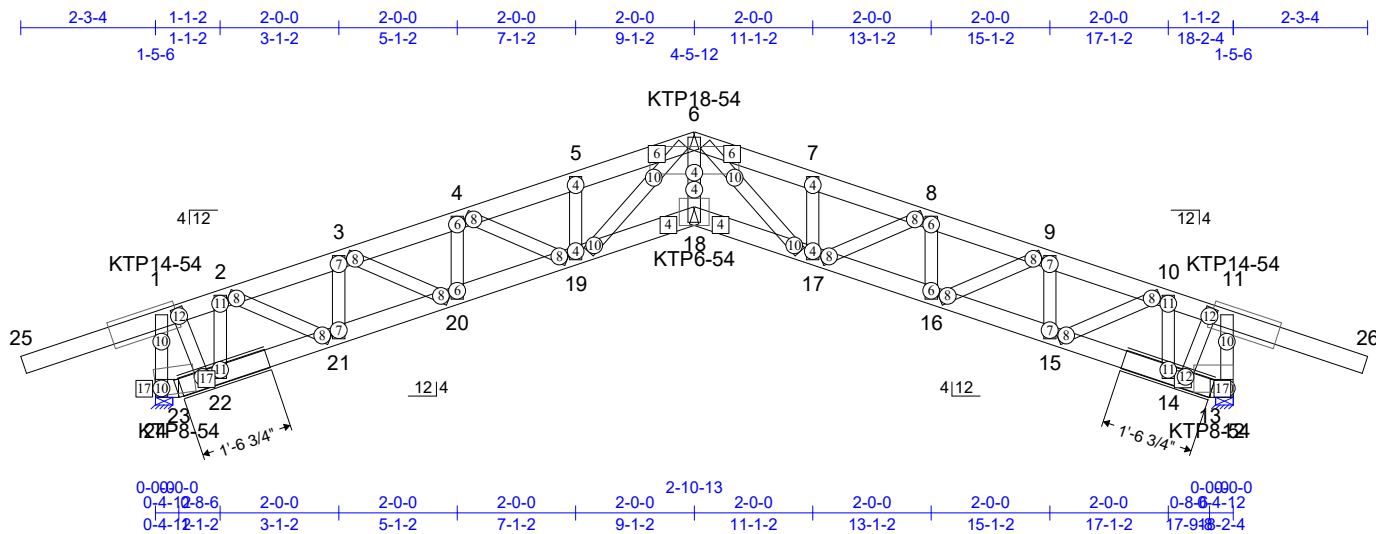
Multiple pinned bearings exist.



4001 Hwy 153
Greenville, SC 29611
864 999 2020

Truss: T03-S2
JobName: 24188
JobID: Jackson County Airport
Date: 5/14/2025 8:36:47 AM
System: KeyTRUSS 6.113
Page: 1 of 2
Report: Eng Plot

SPAN	PITCH	QTY	OHL	OHR	PLYS	SPACING	WGT/PLY
18-24	4/12	1	2-3-4	2-3-4	1	52 in	126.6 lbs



Circles indicate fastener count in webs. Squares indicate fastener count in chords. "Fasteners" indicates the number of #10 SDS (AISI) fasteners required at one end of the member. Each value indicates the number of fasteners required. Where connection plates are called out on this drawing, a plate is required. Refer to General Notes for further clarification. Allowable shear per fastener is calculated per the AISI S100. Maintain fastener spacing at 9"16" min. Maintain fastener edge margin at 9"16" min for each sheet of steel connected.

TC: 0.944(11)
BC: 0.699(22-23)
Web: 0.784(10-14)

TL: 0.06 in
LL: 0.02 in
Cant/OHTL: 0.03 in
Cant/OHLL: 0.02 in
Horz TL: 0.04 in

L/
L/999
L/999
2L/999
2L/999

(loc)
(19-20)
(19-20)
25
26
4

L/240
L/360
2L/180
2L/180

JT	Type	Brg Combo
24	PIn(WL)	1
12	PIn(WL)	1

Note HIGH horizontal reaction

TChd	362S162-68 (50 ksi)
BChd	362S162-68 (50 ksi)
Web	250S162-33 (50 ksi)

TChd	362SI62-68 (50ksi)			TChd Bracing: Sheathed
BChd	362SI62-68 (50ksi)			BChd Bracing: 106 in
Web	250SI62-33 (50ksi)	except:		
	250SI62-54 (50ksi)	1-22	250SI62-54 (50ksi)	11-14 250SI62-54 (50ksi)

Bold lines indicate track reinforcement is required on the hard side of the cee. Track shall match gauge/depth of cee material.

Track shall be attached with fasteners through track into cee using:

- 1 row 12" o.c. through cee flange minimum 3 fasteners per row
- 1 row 12" o.c. through the web minimum 3 fasteners per row

At joints, fasteners that connect webs to chords may be counted as track reinforcement fasteners.

TChdBracing: Sheathed
BChdBracing: 108 in

11-14 250S162-54(50ksi)

11-12 250S162-54(50ksi)

1) This truss has been designed in accordance with IBC - 2018.

2) This truss has been designed for the effects due to wind loads in accordance with ASCE7 - 16, $V = 110$ mph, Exposure C. The building is Gable with Risk Category II, Mean Roof Height 20 ft, Overall Building Dimensions of 67 ft x 131 ft, Enclosure Class: Enclosed, CC Zone Width 6.7 ft. The left end vertical has been exposed to wind. The right end vertical has been exposed to wind. This truss is not an End Zone Truss. This truss is in a Hurricane Prone Region.

3) This truss has been designed for the effects of balanced and unbalanced snow loads in accordance with ASCE7 - 16, using a ground snow load of 5 psf and terrain category B. The exposure condition is Fully Exposed. The thermal condition is All Others ($C_t = 1.0$) with building category II ($I = 1.0$). The roof ventilation is Unknown with Roof R-Value Unknown. The roof slope is 4/12 and the surface type is All Others. This truss has not been designed to account for the effects of ice dams forming at the eaves.

Load Case Lr1: Std Live Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	Cont		Down	Proj	20psf	20psf	52in
Bot	Cont		Down	Proj	0psf	0psf	52in

Load Case D1: Std Dead Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	Cont		Down	Rake	12psf	12psf	52in
Bot	Cont		Down	Rake	8psf	8psf	52in

User-defined Load Case W10: Drag Right

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	Cont		PRight	Rake	900plf	900plf	

User-defined Load Case W11: Drag Left

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	Cont		PLeft	Rake	900plf	900plf	

Load Combinations

#	Load/Combo	Factor
1	D1	1.000
2	D1+Lr1	1.000
3	D1+S1	1.000
4	D1+S2	1.000
5	D1+S3	1.000
6	D1+0.6W3	1.000

[illegible]

5/14/2025

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Keymark Engineering, Inc.
6707 Winchester Circle, Suite 102
Boulder, Colorado 80301



GS Build® System
KeyTruss®
Version: 5.016 [Build 2] master

TruTek Framing Systems

4001 Hwy 153
Greenville, SC 29611
864 999 2020

Truss:

JobName: T03-S2
JobID: 24188
Date: Jackson County Airport
5/14/2025 8:36:48 AM
System: KeyTRUSS 6.113
Page: 2 of 2
Report: Eng Plot

SPAN	PITCH	QTY	OHL	OHR	PLYS	SPACING	WGT/PLY
18-24	4/12	1	2-3-4	2-3-4	1	52 in	126.6 lbs
7	D1+06W8			1.000			
8	D1+06W9			1.000			
9	D1+06W10			1.000			
10	D1+06W11			1.000			
11	D1+045W3+075Lr1			1.000			
12	D1+045W10+075Lr1			1.000			
13	D1+045W11+075Lr1			1.000			
14	D1+045W3+075Sl			1.000			
15	D1+045W3+075S2			1.000			
16	D1+045W3+075S3			1.000			
17	D1+045W10+075Sl			1.000			
18	D1+045W10+075S2			1.000			
19	D1+045W10+075S3			1.000			
20	D1+045W11+075Sl			1.000			
21	D1+045W11+075S2			1.000			
22	D1+045W11+075S3			1.000			
23	06D1+06W1			1.000			
24	06D1+06W2			1.000			
25	06D1+06W4			1.000			
26	06D1+06W10			1.000			
27	06D1+06W11			1.000			

Member Forces Summary

Table indicates: Member ID, max CSI, max axial force, (max comp force if different from max axial force)

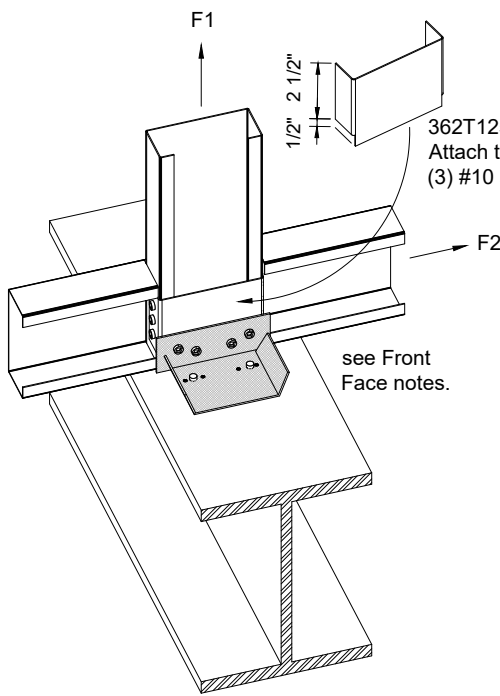
Chd	25-1	0.273	1,393 lbs	(-1,327 lbs)	3-4	0.199	-2,166 lbs	6-7	0.302	-2,164 lbs	9-10	0.188	-1,687 lbs	
	1-2	0.584	-1,105 lbs		4-5	0.188	-2,089 lbs	7-8	0.189	-2,091 lbs	10-11	0.585	-1,105 lbs	
	2-3	0.188	-1,687 lbs		5-6	0.302	-2,167 lbs	8-9	0.199	-2,168 lbs	11-26	0.273	1,393 lbs	
BChd	12-13	0.547	-7,174 lbs		15-16	0.435	-4,255 lbs	18-19	0.142	-1,253 lbs	21-22	0.624	-5,829 lbs	
	13-14	0.699	-6,804 lbs		16-17	0.345	-2,722 lbs	19-20	0.345	-2,727 lbs	22-23	0.699	-6,806 lbs	
	14-15	0.627	-5,826 lbs		17-18	0.141	-1,253 lbs	20-21	0.435	-4,258 lbs	23-24	0.547	-7,176 lbs	
Webs	1-24	0.672	-3,819 lbs		4-20	0.347	1,040 lbs	(-1,032 lbs)	7-17	0.095	-284 lbs	10-14	0.784	-2,313 lbs
	1-22	0.583	4,682 lbs	(-3,306 lbs)	4-19	0.585	-1,643 lbs	8-17	0.585	-1,644 lbs	11-14	0.584	4,684 lbs	
	2-22	0.783	-2,310 lbs		5-19	0.095	-284 lbs	8-16	0.348	1,041 lbs	(-1,033 lbs)	11-12	0.672	-3,819 lbs
	2-21	0.328	1,587 lbs	(-782 lbs)	6-19	0.552	2,257 lbs	(-1,434 lbs)	9-16	0.452	1,587 lbs	(-1,274 lbs)		
	3-21	0.438	-1,298 lbs		6-18	0.237	-704 lbs	9-15	0.438	-1,299 lbs				
	3-20	0.451	1,585 lbs	(-1,271 lbs)	6-17	0.553	2,259 lbs	(-1,436 lbs)	10-15	0.328	1,587 lbs	(-782 lbs)		

Additional Notes:

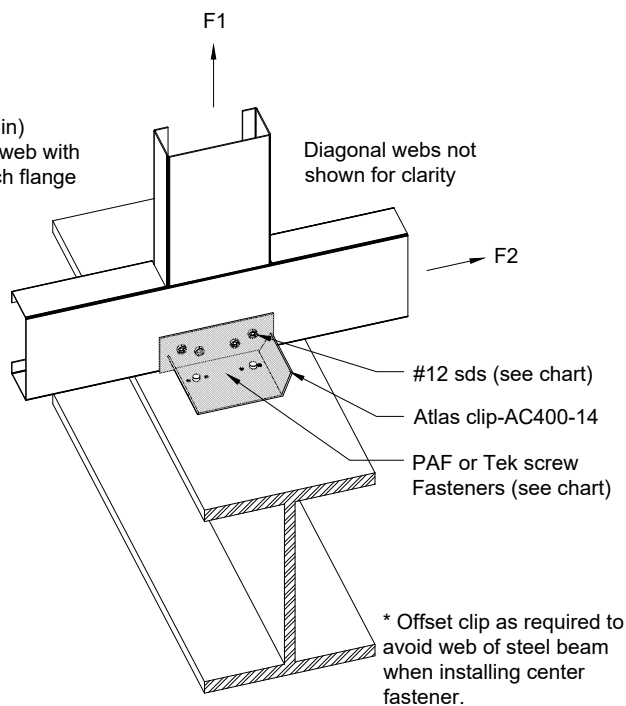
The end of every chord segment shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6 " of the end of each chord segment.

NOTE : Web crippling calculation assume truss is fastened to support.

Multiple pinned bearings exist.



Back Face application
(if/as required)



Front Face application
(typical)

AC400 Clip to Steel Connection Chart

F1 [Front or Back Face]	AC400-14					Truss	AC400-12				Truss
	3/16"	1/4"	3/8"	1/2"	#12 sds		3/16"	1/4"	3/8"	1/2"	
Hilti X-U 0.157	2	942	948	948	948	4	942	1343	1343	1343	5
	3	n/a	n/a	n/a	n/a	n/a	1343	1343	1343	1343	5
#12 Tec	2	942	948	948	948	4	1343	1343	1343	1343	5
	3	n/a	n/a	n/a	n/a	n/a	1343	1343	1343	1343	5
F2 [Front Face]											
Hilti X-U 0.157	2	1924	1924	1924	1924	5 ¹	3468	3468	3468	3468	5 ³
	3	2886	2886	2886	2886	5 ³	3500	3500	3500	3500	5 ³
#12 Tec	2	1122	1122	1122	1122	5	1400	1400	1400	1400	4 ¹
	3	1683	1683	1683	1683	5 ¹	2100	2100	2100	2100	4 ²

¹ - Truss vertical at bearing to be 043 mil minimum.

² - Truss vertical at bearing to be 054 mil minimum.

³ - Truss vertical at bearing to be 068 mil minimum.

F2 [Back Face]

All Bearings ¹		All AC Clips	Truss #12 sds
Truss Vertical	033	184	2
	043	302	2
	054	480	2

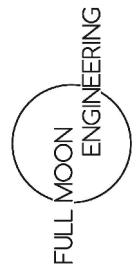
¹ - Minimum connection to bearing as shown above.

General Notes:

- Fasteners to be installed per manufacture recommendations or AISI Standard. Install screws/PAF's/anchors to bearing support through holes or location marks in large leg of connector.
- Equivalent fasteners may be substituted for those specified.
- Welding per ASW (D1.1)
- Detail follows AISI S100 and S240:
 - Minimum SDS spacing = 5/8"
 - SDS shear and tension values based on AISI S100-16.
 - Minimum PAF spacing = 1"
 - Weld transverse and longitudinal strength values per AISI S100-16.
- Supports shown do not represent or imply approved design.
- If Detail is applied to both faces of a truss vertical then front face and back face chart values may be added together.
- Allowable loads have not been increased for wind or seismic.
- Max. reactions shown are non-concurrent.



2/28/2025



Jackson County Airport Terminal
Jefferson, GA

JOB NAME:

LOCATION:



ISSUE DATE:

02/27/25

REVISIONS:

TruTek Job# 24188

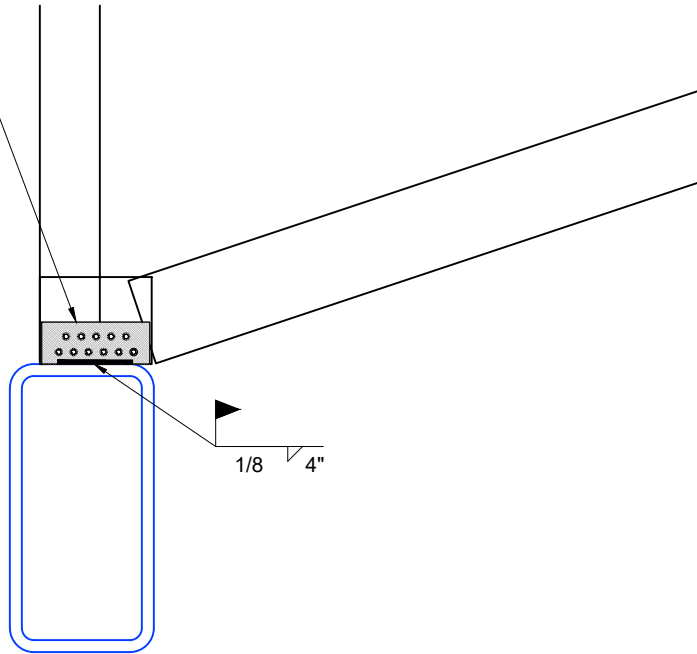
Drawn By: ADT

Approved By: JM

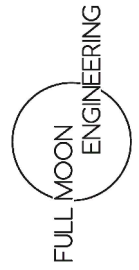
TB1.0

1.75"x5" 97 mil Flat Plate
 -Attach to truss with (11) #12 sds as shown.
 -Attach to structural steel with weld as shown

Max reaction: 7,200 lbs.



Truss bottom chord = 068 mil



Jackson County Airport Terminal

Jefferson, GA

JOB NAME:

LOCATION:



ISSUE DATE:

02/27/25

REVISIONS:

05/14/25

TruTek Job# 24188

Drawn By: ADT

Approved By: JM

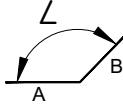
TB1.1

General Notes:

1. Fasteners to be installed per manufacture recommendations or AISI Standard.
2. Equivalent fasteners may be substituted for those specified.
3. Welding per ASW (D1.1)
4. Detail follows AISI S100 and S240:
 - a. Minimum SDS spacing = 5/8"
 - b. SDS shear and tension values based on AISI S100-16.
 - c. Weld transverse and longitudinal strength values per AISI S100-16.
4. Supports shown do not represent or imply approved design.
5. Allowable loads have not been increased for wind or seismic.
6. Max. reactions shown are non-concurrent.



5/14/2025


BENT PLATE SCHEDULE (figured with 6" overlaps)									
QTY	LABEL	Ga.	LENGTH	LEG (A)	LEG (B)	∠ °	PROFILE	DESCRIPTION	
12	RP3	14	10'-0"	4"	4"	152	^	Ridge	
9	RP4	14	10'-0"	4"	4"	144	^	Ridge	
17	FT4	14	10'-0"	3"	3"	72	✓	Fascia-Top	
26	FT3	14	10'-0"	4"	4"	76	✓	Fascia-Top	
21	BP90	14	10'-0"	3"	3"	90	L	Rake	
8	DT90	14	10'-0"	4"	4"	90	L	Deflection Track	
18	FP1	14	10'-0"	8"		180	—	Flat Plate	
SHEAR BLOCKING PLATES									
QTY	LABEL	Ga.	LENGTH	LEG (A)	LEG (B)	LEG (C)	∠ °	PROFILE	
58	SB1	12	3'-4"	2"	13.25"	5"	76	1	
10	SB2	16	3'-4"	2"	6.75"	5"	72	1	
SEPARATE PRODUCTS LIST									
CONNECTORS									
QTY	LABEL	LENGTH	DETAIL	DESCRIPTION					
140	AC400-14	0'-4"	TB1.0	AC400 14g Atlas Truss-to-Bearing Clip					
10	1.75x500-12	0'-5"	TB1.1	1 3/4" x 5" 12ga Truss-to-Bearing Weld Plate					
110	AL362	0'-4"	BB1, BB2/BL1.0	AL362 Stiff Clip					
22	4x1.5x6-90°	0'-6"	SB3/BL1.0	4" x 1 1/2" x 6" Bent 90° 14ga clip					
22	4x1.5x6-108°	0'-6"	SB3/BL1.0	4" x 1 1/2" x 6" Bent 108° 14ga clip					
BRACING									
QTY	LABEL	LENGTH	AREA	DESCRIPTION					
150	150F125-33	5'-0"	CONST	1 1/2" 20ga. Hat-Channel (figured at +/-10' on center)					
70	150F125-33	12'-6"	B.C.	1 1/2" 20ga. Hat-Channel (see BL1.0)					
MISCELLANEOUS									
QTY	LABEL	LENGTH	DETAIL	DESCRIPTION					
25	OL1	5'-9 1/2"	OUTLKR	600S162-43 Outlooker					
16	OL2	6'-4 3/4"	OUTLKR	362S162-43 Outlooker					
8	OL3A	2'-1 1/4"	OUTLKR	600S162-43 Outlooker					
8	OL3B	6'-8 1/2"	OUTLKR	600S162-43 Outlooker					
12	OL362	3'-10"	OUTLKR	362S162-43 Blocking between outlookers					
34	OL600	3'-10"	OUTLKR	600S162-43 Blocking between outlookers					
9	362T125-43	10'-0"	OUTLKR	Cap Track (each end of Outlookers)					
30	600T125-43	10'-0"	OUTLKR	Cap Track (each end of Outlookers)					
100	3x1.5x2.5-18	0'-2 1/2"	OUTLKR	18ga. Clips (each end of Outlooker Blocking)					
16	AL362	0'-4"	4, 8/TL2.0	AL362 Stiff Clip - Outlooker to beam Connection					
10	250S162-43	10'-0"	SB3/BL1.0	2 1/2" 18ga C-stud shear blocking					
10	200STRAP	10'-0"	SB3/BL1.0	2" Wide 18ga. Flat Strap					
32	362S162-43	12'-0"	BB1, BB2/BL1.0	3 5/8" 18ga diagonal and lateral C-stud bracing					
40	HBG8BP0-12X112	0'-1 1/2"	3/TL2.0	1/2"-13 x 1 1/2" Conquest Grade 8 Flange Bolt					
40	HNG2-12HDG		3/TL2.0	1/2"-13 Conquest Grade 2 Hex Nut					
40	ABW12-HDG		3/TL2.0	1/2" Conquest USS Flat Washer					
60	SC62-5/97-KT25		3/TL2.0	Simpson SC62-5/97 Slide-Clip					

JOB NAME:

Jackson County Airport Terminal

LOCATION:

Jefferson, GA



ISSUE DATE:

02/27/2025

REVISIONS:

	05/14/25

TruTek Job# 24188

Drawn By: ADT

Approved By: JM

SP1.0